

# Morphology and Hydrology of a Glacial Stream— White River, Mount Rainier Washington

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GEOLOGICAL SURVEY PROFESSIONAL PAPER 422-A



# Morphology and Hydrology of a Glacial Stream— White River, Mount Rainier Washington

By ROBERT K. FAHNESTOCK

PHYSIOGRAPHIC AND HYDRAULIC STUDIES OF RIVERS

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GEOLOGICAL SURVEY PROFESSIONAL PAPER 422-A

*A study of the hydraulic and morphologic  
processes by which a valley train is formed  
by a proglacial stream*



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## SYMBOLS

$a$	Coefficient in $w=aQ^b$	$k$	Coefficient in $v=kQ^m$
$A$	Area of channel cross section	$L$	Stream length
$b$	Exponent in $w=aQ^b$	$m$	Exponent in $v=kQ^m$
$c$	Coefficient in $d=cQ^f$	$n'$	Roughness parameter from modified Manning equation
$d$	Mean depth $=\frac{A}{w}$	$Q$	Discharge in cubic feet per second
$d_{max}$	Maximum depth	$s$	Water surface slope
$D$	Sediment size, intermediate diameter of particle	$v$	Mean velocity $=\frac{Q}{A}$
$D_{50}$	Median diameter	$v_{0.5}$	Current meter velocity, 0.6 depth
$F$	Froude number $=\frac{v}{\sqrt{gd}}$	$v_s$	Float velocity at surface
$f$	Exponent in $d=cQ^f$	$w$	Channel width at water surface
$g$	Acceleration due to gravity	$\gamma$	Specific weight of water
		$\tau$	Tractive force $=\gamma ds$



## PHYSIOGRAPHIC AND HYDRAULIC STUDIES OF RIVERS

### MORPHOLOGY AND HYDROLOGY OF A GLACIAL STREAM—WHITE RIVER, MOUNT RAINIER, WASHINGTON

By ROBERT K. FAHNESTOCK

#### ABSTRACT

This is a study of the processes by which a valley train is formed by a proglacial stream. The area investigated is the White River valley on the northeast flank of Mount Rainier, between the present terminus of Emmons Glacier and the moraine marking the terminal position in 1913. Five square miles of the 7.5-square-mile drainage basin above this moraine are presently covered by active ice.

Measurements of channel characteristics were made in 112 channels developed in noncohesive materials. Channel widths ranged from 0.7 to 60 feet, mean depths from about 0.03 foot to 2.08 feet, and mean velocities from 0.3 to 9 feet per second for discharges of about 0.01 to 430 cfs. The relations between these variables can be expressed by the equations:  $w=aQ^b$ ,  $d=cQ'$ , and  $v=kQ^m$ . The exponents for White River channels were found to be similar to the average of those for streams in the Southwestern States. In contrast, Brandywine Creek, Pa., with cohesive bank materials, had higher velocity exponents and extremely low width exponents. Width and depth of channels in noncohesive materials may change by scour and deposition as well as by flow at different depths in predetermined channels. White River channels, with steep slopes in coarse noncohesive materials, were narrower, slightly shallower, and had much higher flow velocities than the channels of Brandywine Creek in cohesive materials.

Slope of the valley train was related to particle size and discharge. Pebble counting demonstrated a systematic decrease of 60 mm in median diameter of the valley train deposits in a distance of 4,200 feet downstream from the source areas. Discharge was essentially constant through this reach, the stream received no major additions. Discharges of 200 to 500 cubic feet per second were capable of transporting almost all sizes of materials present and thus modified the form of the valley train.

Data on the velocities required to transport coarse materials in White River showed that a curve in which diameter is proportional to velocity to the 2.6 power approximates the relation better than the traditional sixth power law in which diameter is proportional to velocity to the 2.0 power. The few samples contained suspended-load concentrations up to 17,000 ppm.

The most graphic evidence of the large amount of material transported by the White River was the amount eroded and deposited on the valley train itself. Measurements indicated an average net increase in elevation of 1.2 feet during 1958 and a net decrease of 0.12 foot in 1959.

Description and analysis of the change in pattern of the White River were difficult at high flows because of the rapidity of the change. However, a marked change from a meandering pattern

to a braided pattern took place with the onset of the high summer flows and the pattern returned to meanders with the low flows of fall.

Explanations offered in the literature for the cause of braided patterns include erodible banks, rapid and large variation in discharge, slope, and abundant load. The common element in all explanations seems to be a movement of bed load in such quantity or of such coarseness that there is deposition within the channel, causing the diversion of flow from one channel into other channels in a valley wide enough to provide freedom to braid. White River braiding took place most actively at large loads and discharges.

Although examples of braiding by an aggrading stream are common, the White River and the Sunwapta River, Alberta, have reaches in which degradation took place while the stream had a braided pattern. The conclusion is reached that both braided and meandering reaches can occur along the same stream, which may be aggrading, poised, or degrading. The pattern alone does not conclusively define the regimen of the stream.

The regimen of the glacier has long-term effects in providing debris to the stream; short-term effects of weather and runoff determine the current hydraulic characteristics, rate of deposition and erosion, and channel pattern.

#### INTRODUCTION

This is a study of the processes by which a valley train is formed by a proglacial stream. The term valley train, as used here, is an outwash plain laterally constricted by valley walls. Studies of such distinctly different geologic environments improve the understanding of past geologic events and knowledge of the interrelations of hydraulic and morphologic variables. Comparisons of data from distinctly different environments make possible the evaluation of the sensitivity of hydraulic parameters to environment.

It is hard to imagine a more radical departure from normal stream regimen than a glacierized drainage basin (fig. 1). Diurnal fluctuations in discharge bring bankfull or overbank flow for brief periods to many short reaches of the stream. As a large part of the precipitation falls as snow, it may be years before heavy accumulations at high elevations are reflected in changes of position of the glacier and in runoff.

Most of the runoff occurs during the months of June, July, and August, which in many environments would be the period of extreme low water. The presence of the Emmons Glacier makes it possible to study the relation of stream and glacial regimen. The stream pattern changes rapidly in response to changes in discharge which cause much shifting of debris on the valley train. The coarseness of the materials being transported, the high stream gradient, and the rapid flow provide situations in which the influence of these factors can be measured under extreme conditions.

Hjulström (1935) and Sundborg (1954) in their detailed studies of river systems summarized contemporary knowledge of sediment transport and open-channel hydraulics and applied this knowledge to natural channels, which had low velocities, tranquil or subcritical flow, relatively fine bed materials, and low slopes. Leopold and Maddock (1953) presented a method of quantitative analysis of channel characteristics of natural streams. They limited their discussion to a number of rivers in the Great Plains and the Southwest. Wolman (1955) applied the method to a stream in a more humid region. In a recent investigation where these methods were used, Miller (1958) studied high mountain streams in regions which had at one time supported glaciers. All of these channels differed from laboratory channels and from the rapid streams which issue from glaciers to flow with steep gradients and constantly changing channel patterns to the sea. Hjulström led an expedition during the summers of 1951 and 1952 to study the alluvial outwash plains (sandurs) of Iceland and the mechanics of braided streams. The stream studied by Hjulström is much larger and has more gentle gradients than the White River and pattern changes appear to take place much less rapidly than on the White River valley train.

The first section of this report describes the regional setting and the recent history of the Emmons Glacier. The second and third sections cover the hydrology and hydraulic characteristics of White River channels and the transportation, erosion, and deposition of materials of the valley train. The fourth section is a description and analysis of the channel pattern change on the valley train. The fifth section is a discussion of the related problems, the application of the concept of equilibrium to valley train formation and the influence of the glacier regime on valley train formation.

The present report is a modification of a thesis<sup>1</sup> submitted to Cornell University. Work in the field was performed by the author as a member of the Geological Survey under the general supervision of C. C. McDonald, Chief, General Hydrology Branch. The

field assistants were T. G. Bond in 1958 and P. V. D. Gott in 1959. Cornell University professors Marvin Bogema, R. A. Christman, P. G. Mayer, and C. M. Nevin, graduate committee members; E. H. Muller and L. L. Ray, committee chairmen; and M. G. Wolman, U.S. Geological Survey, made many helpful suggestions in regard to the fieldwork and the manuscript.

John Savini, U.S. Geological Survey, was extremely helpful in the field through his aid with problems of stream gaging and in preparation of streamflow records. Ann M. Fahnestock, the author's wife, provided material support in the fieldwork, maintenance of camp, and preparation of the original manuscript.

## DESCRIPTION OF STUDY AREA

### PHYSICAL FEATURES

The White River study area lies within Mount Rainier National Park on the northeast flank of Mount Rainier (figs. 1 and 2), west of the crest of the Cascade Mountains, 80 miles south southeast of Seattle, Wash. The area of most intensive study was the 1-mile reach of the stream between the present terminus of the Emmons Glacier and the valley loop moraine, which marks the 1913 terminus. The study area is reached from the White River Campground by a truck trail leading up the left bank of the Inter Fork and by a trail across the moraines. This area was selected because it is accessible and includes both a site for a gage where the stream is confined to one channel and a reach where at times the channel pattern changes rapidly. Measurements to determine the relation of size of material and slope were made in several other areas.

An area of 7.5 square miles (M. F. Meier, written communication, 1958) is tributary to the gage at the moraine (fig. 2). Of this area, 5 square miles was covered by active glacier ice, 4.4 square miles by the Emmons Glacier, and 0.6 square mile by that part of the Frying Pan Glacier thought to contribute to the White River above the gage. Flow from the Frying Pan Glacier, which does not pass through the study area, enters the White River about 3 miles below the gage by way of Frying Pan Creek.

In the period 1957-59 the Emmons Glacier (fig. 2) extended from its source area on the summit of Mount Rainier at an elevation of more than 14,000 feet to an elevation of about 5,300 feet through a valley carved into the northeast flank of the mountain. The depth of this erosion is suggested by the following data: The surface of the glacier is about 2,000 feet lower than the summit of Little Tahoma Peak (fig. 1) on a line transverse to the valley; and the terminus in 1910 was at an elevation of about 4,700 feet near the present stream-

<sup>1</sup> Fahnestock, Robert, K., 1960, *Morphology and Hydrology of a glacial stream: Ithaca, N. Y., Cornell University, doctoral thesis.*





FIGURE 1.—Emmons Glacier and the White River study area, Mount Rainier. Photograph by M. F. Meier, October 1, 1958.



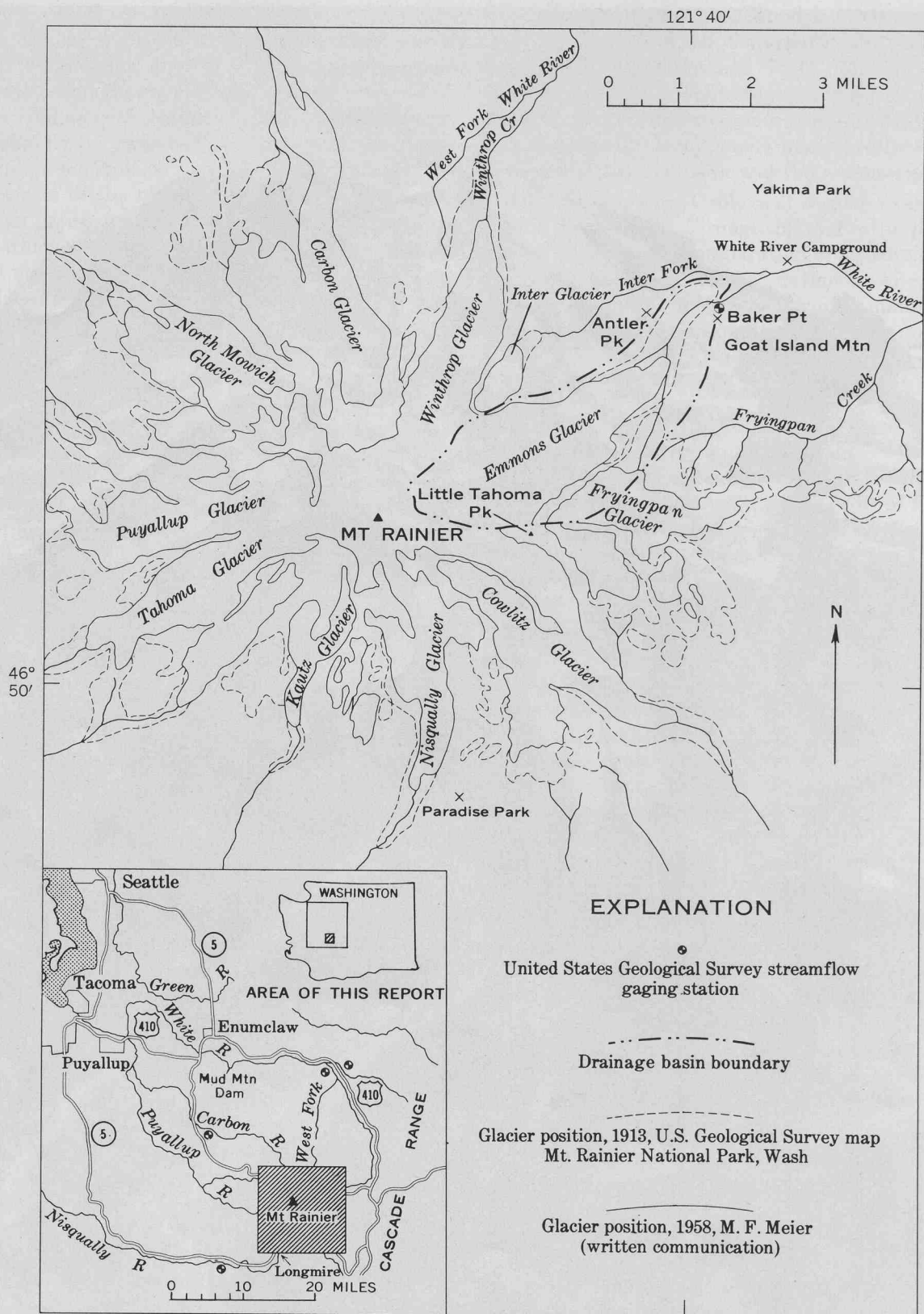


FIGURE 2.—Study area and distribution of active glaciers on Mount Rainier, 1913 and 1958.

gaging station, which is some 1,700 feet lower than the adjacent Yakima Park, Baker Point, and Antler Peak (fig. 2). The White River is tributary to Puget Sound through the Stuck and Puyallup Rivers.

A stream-gaging station equipped with a water-stage recorder is 1 mile below the glacier at the moraine; another station, White River at Greenwater, Wash., is 28 miles downstream, just above the junction with the Greenwater River and 2 miles below the junction with the West Fork White River. The source of the West Fork White River is the Winthrop Glacier, which emanates from the same icefield as the Emmons Glacier.

#### CLIMATE AND VEGETATION

The position of Mount Rainier on the west side of the Cascade crest is of great climatic significance. The prevailing winds, moisture-laden westerlies from the Pacific Ocean, are cooled as they rise to cross the Cascade Mountains, causing heavy precipitation. The proximity of the Pacific Ocean has a moderating effect on the temperature in both summer and winter.

On Mount Rainier, approximately 75 percent of the precipitation falls from October through May. Yearly snowfall at the higher elevations of the mountain has not been measured, but records kept on the southwest side of the mountain at Paradise (elev 5,400 ft) indicate an average precipitation of 100 inches (including 50 ft of snow) and at Longmire (elev 2,160 ft) approximately 78 inches (including 15 ft of snow). Brockman (1947, p. 2) noted that "while no records are available for Yakima Park (northeast side of the mountain) . . . the snowfall is considerably less." The maximum depth of snow on the ground at Yakima Park is from 40 to 50 percent that at Paradise, which is almost 1,000 feet lower.

The vegetation of Mount Rainier National Park varies with elevation from that of the humid transitional zone of the Puget Sound Lowland to that of the Arctic-Alpine Zone above 6,500 feet (Brockman, 1947). However, in the area studied there is no vegetation on the valley train owing to the incessant reworking of the surface. Scattered trees as much as 57 (Sigafos and Hendricks, 1961, p. 4) years in age grow on the adjacent ablation moraine, which is underlain by stagnant ice. Older trees are found on the adjacent lateral moraines. The vegetation is too sparse to have any appreciable effect on the runoff characteristics of the drainage basin. On the valley train below the 1910-13 moraine, however, alder has found a footing and scattered groups of evergreen and poplar trees are growing. Older trees grow on terraces, the lowest of which is about 2 to 5 feet above the present valley train. Some trees on channel islands and banks have been killed where

deposition has raised the bed of the stream above its former banks.

#### GEOLOGY

The bedrock formations of the drainage basin range widely in appearance, composition, and occurrence. These formations provide the glacier, and thus the stream, with rocks having a wide range of abrasion resistance and specific gravity. The average specific gravity of 29 specimens from the ablation moraine was 2.5, ranging from 1.9 to 3.5 as determined in the field by means of a spring scale.

The lavas, agglomerates, and mudflows of the Mount Rainier volcanics of Coombs (1936) lie on a rugged erosion surface cut into the Keechelus andesite series. According to Coomb's (1936) description of the rocks and their occurrence, the Keechelus andesitic series, composed of massive tuffs, breccias, and porphyries with subordinate andesite flows, felsites, basalts, hornfels, and sediments, was intruded by the Snoqualmie granodiorite. The reaction zone produced by this intrusion is the source of additional rock types. A more detailed examination of the geology of Mount Rainier National Park is currently (1959) being made by Waters, Hobson, and Fisk of Johns Hopkins University.

Emmons Glacier and the White River have cut through the Mount Rainier volcanics into the Keechelus andesitic series, but the distribution of these formations is obscured by the presence of the glacier and its deposits, and by the large areas of talus on the slopes above the most recent lateral moraines. The Snoqualmie granodiorite is not known to crop out upstream from the gaging station, but some of the rocks found in the valley train may be from the reaction zone between the granodiorite and the Keechelus. Apparently the varied lithology of the bedrock does not influence the present longitudinal profile of the stream, for only two small patches of bedrock, neither of which is in the present streambed, are known to crop out within the valley train. Variations in the resistance of the bedrock to erosion by the glacier are apparent in the domes and hollows of the glacier surface and the resulting crevasse pattern of the glacier (fig. 1).

An unusual event in the geologic history of the northeast side of the mountain is the volcanic mudflow (greater than 0.25 cubic mile) described by Crandell and Waldron (1956). It issued from the northeast flank of the mountain and flowed down both forks of the White River, spreading out in the Puget Sound Lowland to points beyond Enumclaw, some 45 miles downstream. Carbon-14 analyses of wood samples from the mudflow have dated it at about 4,800 years ago.

The glacial geology of the White River valley has been studied by D. R. Crandell, R. D. Miller, D. R. Mullineaux, and H. H. Waldron of the U.S. Geological Survey, but their findings are not yet published. Evidence of the extent of the Emmons Glacier in Pleistocene time is the glaciated character of the White River valley for many miles downstream. Crandell and others (oral communication) have found a number of lateral moraines at various elevations within the valley and are attempting to correlate them with deposits in other valleys.

The massive plug of debris that blocks the valley about 1 mile below the glacier terminus is evidence of the extent of the glacier at intervals over the last 1,000 years. It is composed of several moraines of different ages. The dating of these moraines by tree-ring count is reported in Sigafoos and Hendricks (1961). Crandell (oral communication) and Sigafoos and Hendricks found that trees about 60 to 80 years old are growing on the innermost stabilized moraine; whereas, less than 100 feet toward the glacier, trees about 50 years old are growing on an ablation moraine on top of stagnant ice.

#### EMMONS GLACIER

The study area has been uncovered by the stagnation and melting of the Emmons Glacier since its position was recorded by some of the earliest observers. One of the first authors to record his observations on the "White River Glacier" as it was then called, was S. F. Emmons, who with A. D. Wilson made the second successful ascent of the mountain in 1870. Emmons in a letter to his chief (King, 1871) seems to have been overenthusiastic and exaggerated the glacier's dimensions, stating:

The main White River glacier, the greatest of the whole, pours straight down from the rim of the crater in a northeasterly direction, and pushes its extremity farther out into the valley than any of the others. Its greatest width on the steep slope of the mountain must be four or five miles, narrowing towards its extremity to about a mile and a half; its length can be scarcely less than ten miles.

The map of Mount Rainier and its glaciers by Sarvent and Evans dated 1896, (plate 68, in Russell, 1898, p. 363) shows the Emmons Glacier terminus in approximately the same position as shown by the 1913 U.S. Geological Survey map (fig. 2). The 1896 map, however, shows two streams issuing from the glacier—one as shown on the 1913 map and the second flowing into the Inter Fork at the left side of the glacier terminus. F. E. Matthes (1914) stated:

The youngest moraine, fresh looking as if deposited only yesterday, lies but 50 feet above the glaciers' surface and a scant 100 feet distant from its edge; the older ridges subdued in outline and already tinged with verdure lie several hundred feet higher on the slope.

Matthes gave the length of the Emmons Glacier as about  $5\frac{1}{2}$  miles and its width as  $1\frac{1}{4}$  miles in its upper half. The area of active ice, measured from the 1913 map as approximately 5.3 square miles, had decreased to 4.4 square miles in 1958 (M. F. Meier, written communication). Most of the 0.9 square mile decrease represents areas of ablation moraine and valley train underlain by slowly melting stagnant ice. Matthes' (1914) estimate of 8.5 square miles could not be verified by rechecking with the 1913 map.

Periodically since 1930 the U.S. National Park Service has studied the position of the glacier terminus; the data show the rate of recession of the point at which the stream emerged from under the glacier to be about 75 feet per year. The positions of the glacier (fig. 3) are sketched from Park Service data and photographs provided by V. R. Bender, park naturalist. In 1943 an ice tunnel was discovered to have caved in upstream from the ice face being measured; the caving produced a second point from which the ice faces receded both upstream and downstream. This second point was near the present junction of East and West Emmons Creeks; thus, an ice mass was left bridging the valley until at least 1953.

Rigsby (1951) spent 6 weeks in 1950 studying ice petrofabrics and glacier motion on the Emmons Glacier. He determined the rate of motion to be as much as 0.75 foot per day in the center of the glacier, about half a mile above the position of the terminus in 1958 (fig. 2).

The general advance of glaciers of the Cascade Mountains (Hubley, 1956, and Harrison, 1956a and 1956b) had also started on the Emmons Glacier by 1953. The National Park Service has measured this advance periodically since 1953 and has found that it averaged about 165 feet per year from 1953 through 1957 and continued in 1962. In 1957 Arthur Johnson, of the U.S. Geological Survey, began taking a yearly series of phototheodolite pictures from which topographic maps of the glacier might be made by terrestrial photogrammetry. Volumetric computations as well as measurements of the change in position of the glacier terminus will be possible when these maps are available.

The present location of the valley train is in the approximate position of the clear ice shown by the 1913 U.S. Geological Survey map. The debris-covered ice along the valley train appears to have changed little in elevation since the 1913 map, emphasizing the role played by the debris in insulating the ice along the sides of the glacier tongue. The approximate elevation change of the clear ice is shown in figure 4. Downwasting is almost overshadowed by the combination of caving and melting, and by erosion and melting from below by streams of water and air that flowed through ice tunnels in this central part of the glacier. In 1959,



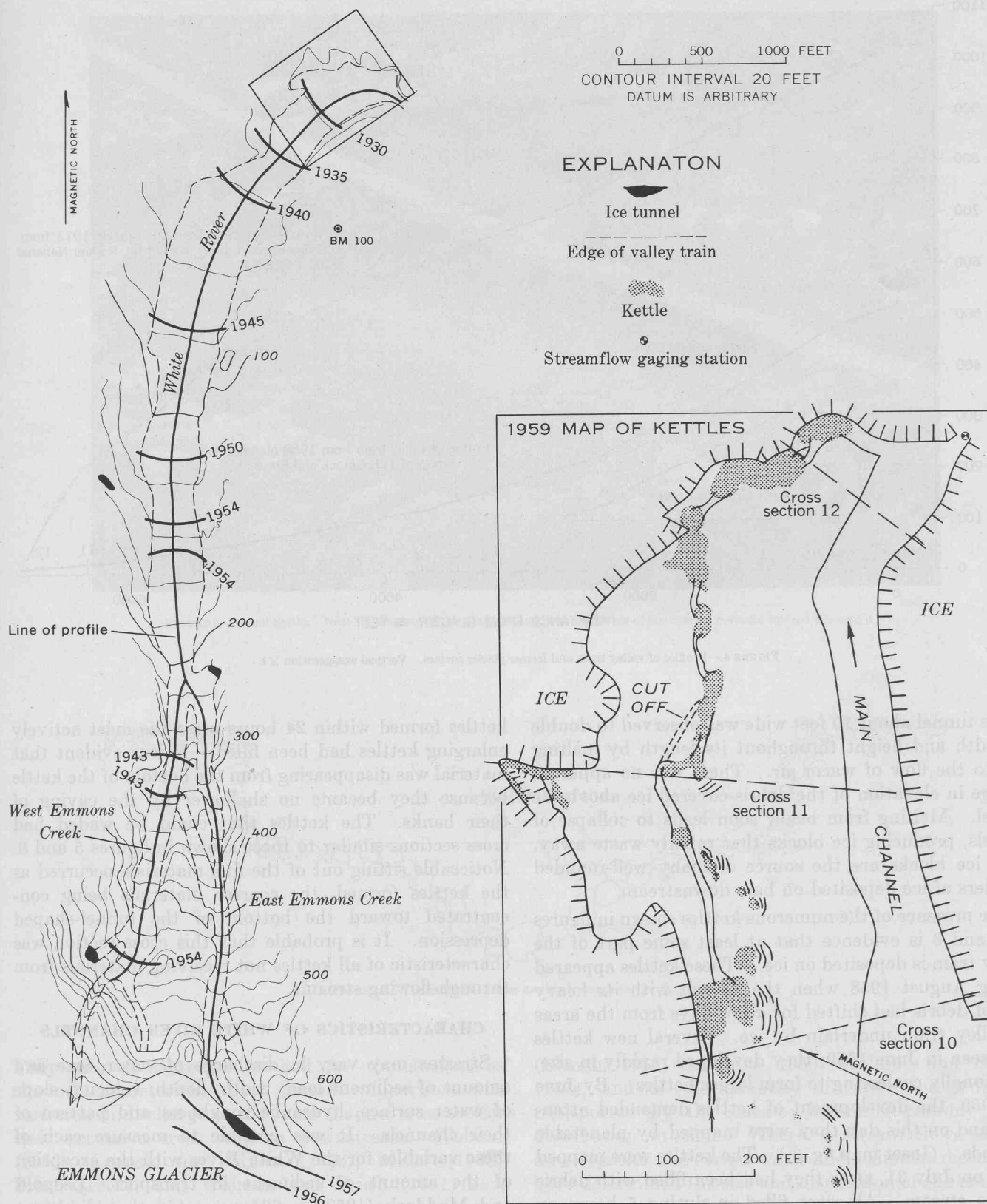


FIGURE 3.—Map showing approximate positions of Emmons Glacier ice front, 1930-58, from National Park Service glacier surveys, line of profile (fig. 4), and inset map showing kettles in 1959.

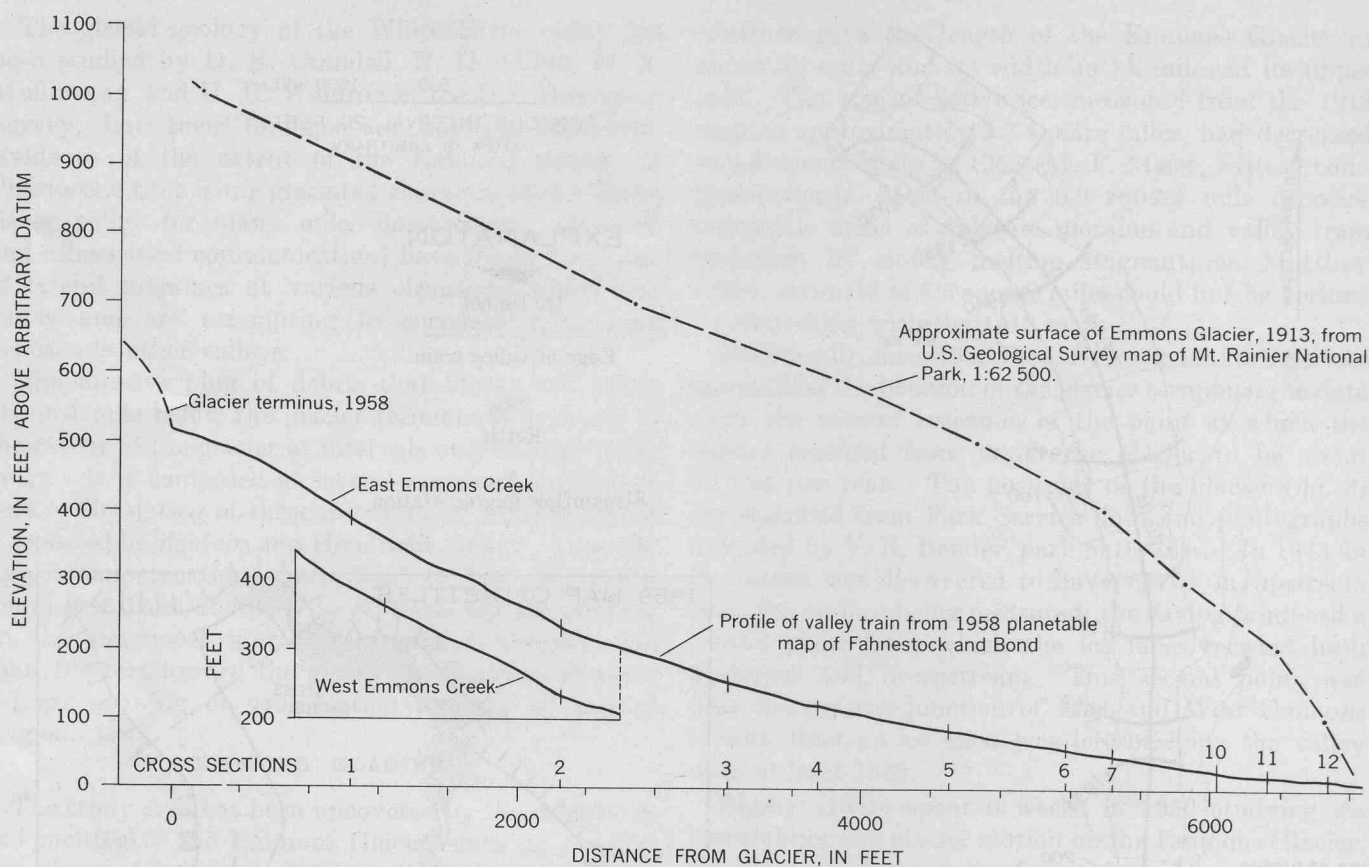


FIGURE 4.—Profiles of valley train and former glacier surface. Vertical exaggeration  $\times 4$ .

an ice tunnel about 10 feet wide was observed to double in width and height throughout its length by melting due to the flow of warm air. There was no apparent change in elevation of the debris-covered ice above the tunnel. Melting from below soon leads to collapse of tunnels, producing ice blocks that rapidly waste away. Such ice blocks are the source of many well-rounded boulders of ice deposited on bars downstream.

The presence of the numerous kettles shown in figures 3, 5, and 6 is evidence that at least some part of the valley train is deposited on ice. These kettles appeared during August 1958 when the stream with its heavy load of debris had shifted for a few days from the areas of valley train underlain by ice. Several new kettles were seen in June 1959; they developed rapidly in size, occasionally coalescing to form larger kettles. By June 25, 1959, the development of kettles demanded attention, and on this day they were mapped by planetable methods. (Inset map fig. 3.) The kettles were mapped again on July 31, after they had been filled with debris by the stream. All were filled in during 5 hours on July 20, 1959, with the exception of one which the stream did not reach. This kettle was filled in a few hours on July 23. Cracks in the ground and incipient

kettles formed within 24 hours after the most actively enlarging kettles had been filled. It was evident that material was disappearing from the bottom of the kettle because they became no shallower by the caving of their banks. The kettles that could be waded had cross sections similar to those shown in figures 5 and 6. Noticeable sifting out of the fine materials occurred as the kettles formed, the coarser materials being concentrated toward the bottom of the funnel-shaped depression. It is probable that this cross section was characteristic of all kettles not receiving materials from through-flowing streams.

#### CHARACTERISTICS OF WHITE RIVER CHANNELS

Streams may vary in discharge of water, size and amount of sediment load, width, depth, velocity, slope of water surface, hydraulic roughness and pattern of their channels. It was possible to measure each of these variables for the White River with the exception of the amount of sediment in transport. Leopold and Maddock (1953, p. 33) suggested that discharge and sediment load are the result of the climatic and geologic environment within the basin and that the other variables adjust to these imposed conditions.

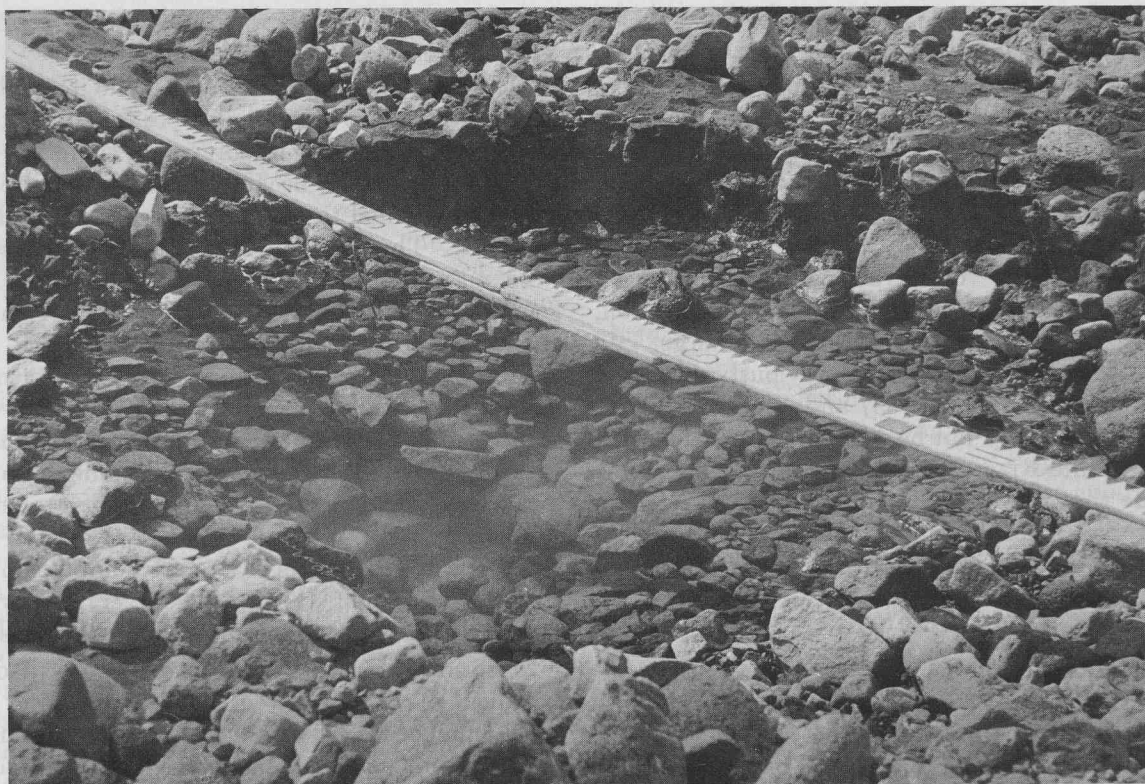


FIGURE 5.—Small kettle. Note the absence of finer material and concentration of the coarser material toward the center.

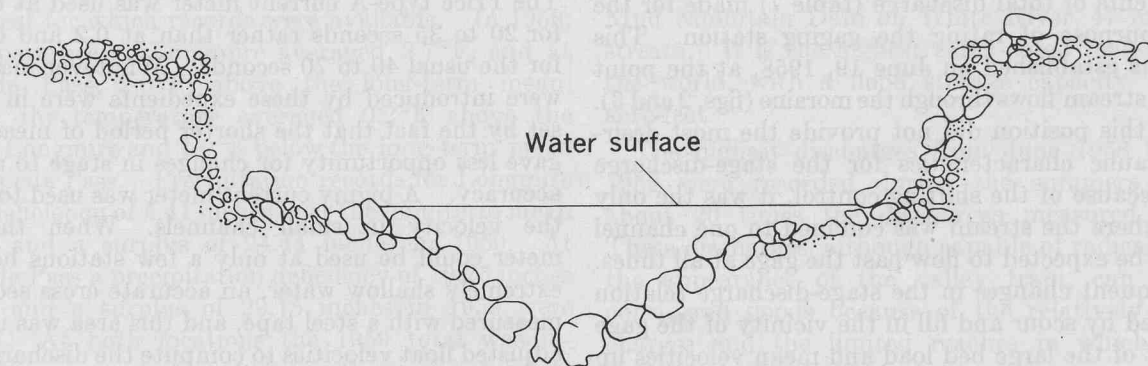


FIGURE 6.—Diagrammatic cross section through kettle, figure 5.

They also presented a method for the analysis of channel characteristics that permits comparisons with variation in discharge at the same channel cross section and successive cross sections downstream. Wolman (1955), Leopold and Miller (1956), Miller (1958), Brush (1961), and Wolman and Brush (1961) have applied this method to the analysis of a variety of natural streams and flume channels.

The shape of a channel is reported to be a function of the type of materials that make up its bed and banks

(Blench, 1956; Schumm, 1960; Wolman and Brush, 1961), and of the quantity of water and sediment transported by the stream, including possibly their distribution in time. Where a stream flows on its own deposits, the earlier water and sediment discharges have determined the size and disposition of the material composing the bed and banks. Streams will adjust in different ways to increases or decreases in discharge of sediment and water, in accordance with the character of bank materials. It is thought that two streams



with the same mean annual discharge will develop somewhat different channel characteristics if the flow distribution for one is uniform throughout the year and for the other is the product of periodic flooding.

The bed and bank materials of the White River, to be described in detail later, are deficient in silt- and clay-size materials and are therefore noncohesive and relatively easily eroded. They provide abundant bed and suspended loads. Observations indicate that a change in the channel shape by erosion or deposition, or both, accompanies any change in discharge. As this was not true for those streams previously described by Leopold and Maddock (1953) and Wolman (1955), some of their data may not be strictly comparable with those of the White River. In spite of this, comparisons can be made that reflect and help define the differences in characteristics of channels in readily erodible materials and channels in more resistant materials.

#### DISCHARGE

Measurements of channel characteristics were made in 112 channels that were selected as representative of a wide variety of channel sizes and shapes. It was necessary to make discharge measurements with reasonable accuracy—a criterion that eliminated channels in which bed-load movement was extremely heavy and which were rapidly changing their shape. Of these representative channels, 81 were included in the 48 measurements of total discharge (table 7) made for the primary purpose of rating the gaging station. This station was established on June 19, 1958, at the point where the stream flows through the moraine (figs. 2 and 3). Although this position did not provide the most desirable hydraulic characteristics for the stage-discharge relation because of the shifting control, it was the only position where the stream was confined to one channel and could be expected to flow past the gage at all times.

The frequent changes in the stage-discharge relation were caused by scour and fill in the vicinity of the gage as a result of the large bed load and mean velocities up to 9 feet per second. Frequent measurements of discharge were required to compute these changes.

Without doubt the variation in the stage-discharge relation introduced inaccuracies in discharges determined from the recorded stage, but the frequency of discharge measurements and adjustment of the stage-discharge relation probably kept these errors within reasonable limits. The adjustments were made by John Savini according to standard Geological Survey methods for gaging streams with shifting control. He utilized numerous discharge measurements, temperature records, and records from the nearest gaging station, White River at Greenwater, Wash., as well as records from the station recording runoff at South

Cascade Glacier, in the northern Cascade Mountains, to compute the adjustments.

Discharge measurements were made in reaches of the stream where 1 to 4 channels were found. At the time of highest discharge, measurements could not be made in multiple channels because streamflow shifted rapidly from one channel to another during the period of measurement. The channels in which rating measurements were made, although typical of the White River in general, are not representative of some of the widest, shallowest channels, in which much of the flow was around boulders, preventing accurate measurements. These wide, shallow channels were extremely unstable and often divided into several narrower channels.

A number of measurements were made to obtain data for channels with a wide range in discharge and shape. Measurements were made also in very small channels with a discharge of 1 cfs or less, for comparison with the larger channels of the White River and with data obtained for flume channels by Wolman and Brush (1961).

Most measurements of discharge were made by an expedient method, adopted to reduce the wear and tear on equipment and personnel. Several current meters were broken, and feet and shins were bruised by the coarse material being transported by the stream. The velocity and depth were determined at 10 to 15 stations rather than the 20 or more stations customarily used to average out variations in flow across the channel. The Price type-A current meter was used at 0.6 depth for 20 to 35 seconds rather than at 0.2 and 0.8 depth for the usual 40 to 70 seconds. Whatever inaccuracies were introduced by these expedients were in part offset by the fact that the shorter period of measurement gave less opportunity for changes in stage to affect the accuracy. A pigmy current meter was used to measure the velocity in small channels. When the pigmy meter could be used at only a few stations because of extremely shallow water, an accurate cross section was measured with a steel tape, and this area was used with adjusted float velocities to compute the discharge. The mean velocity in a vertical was taken as 0.6 of the float velocity, on the basis of 20 discharge measurements where both current meter and float velocity data were available for the same channel.

The estimated error in current-meter measurements of discharge ranged from 5 percent at moderate discharges to 15 percent at high and low discharges. The accuracy of discharges estimated from gage heights for use in the study of stream pattern depends both on the accuracy of the rating measurements and on the length of time elapsed between such rating measurements and the event for which discharge was estimated. The possible error is as much as 25 percent of the estimated discharge.

The hydrograph (fig. 50) computed by Savini shows the fluctuations in discharge and differences in runoff for the summers of 1958 and 1959. The year 1958 was unusual for the Pacific Northwest because snowfall was light during the winter of 1957-58 and the summer was warm with long spells of clear weather, which produced high ice melt. By midsummer, areas of the mountain at high elevations, which had not been uncovered before in the memory of local inhabitants, were free of snow. Winds blowing over such areas raised a column of dust above the southwestern side of the mountain during July and August. The snow and ice on the mountain were brown from fallen dust, which served to increase melting by lowering the albedo of the snow and ice.

In 1959, snow at all elevations lasted much longer; many areas that were uncovered the previous summer remained under snow. Melt water produced during the summer was considerably less than in 1958 (hydrograph, fig. 50). The average flow for July 1959 was about 20 percent less, for August about 35 percent less, than for the same months of 1958. The variations from the long-term mean for average temperature and total precipitation for the periods October 1957 to September 1958 and October 1958 to September 1959 at Longmire on the southwestern side of the mountain, and at Stampede Pass 20 miles to the north, might be considered representative of the difference in weather in 1958 and 1959. These two stations were the nearest for which records were available. In 1958, the temperature at Longmire averaged 2.7°F, and at Stampede Pass, 2.6°F, above the long-term mean. In 1959, the temperature averaged 0.2°F above the mean at Longmire and 1.0°F below the long-term mean at Stampede Pass. Precipitation records for Longmire show a deficiency of 5.41 inches over the long-term mean in 1958 and a surplus of 24.44 inches in 1959. At Stampede Pass a precipitation deficiency of 19.25 inches in 1958 and a surplus of 29.15 inches in 1959 were recorded. At both locations the 1959 total was increased in an extraordinarily rainy September by 9.72 inches of precipitation at Longmire and 11.20 inches at Stampede Pass.

The order of magnitude of the winter flow given by discharge measurements 21a, 49, and 50 (table 7), made in November 1958 and 1959, is 20 to 40 cfs. The gage well froze in late October and the gaging station remained inoperative for low flows until May, so that these measurements are the only flow data available for this period.

The reasons that the melting snow of April, May, and June do not produce flows at the moraine gage (figs. 2, 3) as high as those produced by the melting of July and August may be the following: (a) Unlike nonglaciated

drainage basins, which usually have lost their snow cover by late June, more than 70 percent of the White River drainage basin above the moraine has a year-round snow and ice cover. The warm weather of the summer months produces rapid melting. (b) Flow continues throughout the winter. It is thought to result from ground-water discharge as well as from subglacial melting, as the daily mean temperature often falls below freezing for periods of several days. Thus, in the spring, ground-water recharge must take place before snowmelt can produce much direct runoff. (c) The albedo of snow is much higher than that of ice (Sauberer and Dirmhirn in Hubley, 1957, p. 77, table 5), so that the same amount of radiation produces more melt water per unit area from exposed ice than from snow and the area of exposed ice enlarges throughout the melt season. (d) Cloud cover is a factor that should be considered, although records are not available for analysis. The shielding from radiation of the heavier cloud cover of spring may well be offset by heavier precipitation. Summer precipitation was slight in both 1958 and 1959.

No records or field evidence of flooding prior to 1957 were found for the study area. As the surface of the valley train is constantly changing owing to lateral shifting of channels and deposition, all evidence might well be obscured a few years after a flood. That floods do occur in the downstream part of the basin is suggested to even a casual observer by the size of Mud Mountain Dam on White River, 47 miles downstream. It is the highest earth-cored rock-fill dam in the world, with a flood storage capacity of 106,000 acre-feet.

The highest discharges from June 1958 to October 1959 were recorded during the summers and were about 20 times the discharges measured in winter. These discharges, although capable of radically altering the appearance of the valley train, can hardly be considered floods because of the relatively small discharges and the limited reaches in which overbank flow occurred.

In the fall and winter of 1959, there were two severe storms at Mount Rainier. On October 22 a rainfall of 5.5 inches in 24 hours was recorded at Paradise on the southwestern side of the mountain. It produced a rise of about 4 feet at the staff gage at Longmire (fig. 2) and a rise of about 1 foot at the station on the White River at the moraine. A second storm on November 22, 1959, produced a rise of about 1.2 feet in the White River; the peak flow was estimated by Savini to be about 1,000 cfs. Where the flow was concentrated in one channel at the gage, boulders estimated to be as large as 8×8×5 feet were undermined and moved; however, photographs taken by Savini 1 week after

this storm show no major changes in the valley train deposits, although some scour is evident in that part of the valley train within 100 yards above the gage. The effects of flooding were much more evident on the Inter Fork, the West Fork, and other tributaries where roads and bridges were washed out.

Surveys run in June 1960 showed that there was several feet of deposition over parts of the valley train. This storm may have produced a greater proportion of overbank flow than usual, but in most of the area the flow was probably contained in definite channels. The frequency of recurrence of such a flow cannot be established with such a short period of record; but on the White River at Greenwater, Wash., 28 miles downstream (fig. 2), with 30 years of record, such a storm was estimated to have a recurrence interval of 45 years. There was a third major storm during December, but its effects were not separable from those of the other storms when the area was revisited in June 1960.

Diurnal fluctuations in discharge for the White River range from an estimated 10 cfs on a winter day to more than 700 cfs after several consecutive warm summer days. Figure 7 is a reproduction of the water-stage recorder chart for July 21–23, 1958, which shows three types of fluctuations recorded by the gage: (1) diurnal, (2) intermediate, (3) minor. The largest is the diurnal fluctuation, which reaches its peak in the late afternoon and its low point in the early morning. Its magnitude, on July 22, was about 0.65 foot. This represents a change in discharge from 140 cfs to approximately 600 cfs. The fluctuations of intermediate size, such as the one at 2 p.m. on July 22, with a magnitude of 0.3 foot and a duration of about 1 hour, could represent either bar formation and removal that temporarily

altered the stage-discharge relation, or periodic storage and release of water from the crevasse system of the glacier. Other more probable explanations for such variations are the storage and release of water by the formation and destruction of antidunes, or a change in bed roughness in the vicinity of the gage similar to that described by Dawdy (1961). The minor fluctuations of 0.1 foot and 5-minute duration, superimposed upon the other fluctuations, are thought to be related to the movement of small amounts of material or to changes in position of standing waves in the control section of the gaging station, although it is possible that some of these minor fluctuations reflect changes in discharge.

#### WIDTH, DEPTH, VELOCITY, AND AREA

In the process of measuring discharge, the width, depth, velocity, and cross-sectional area of flow were determined. Widths ranged from about 0.7 foot at 0.01 cfs to a maximum of 60 feet at 330 cfs (channels 38 and 25, table 7). Mean depths of the White River ranged from 0.03 foot to 2.08 feet for discharges of 0.01 to 430 cfs (channels 38 and 88, table 7). Mean velocities of the White River ranged from 0.3 to 9 feet per second for discharges of 0.01 to 300 cfs (channels 38 and 83, table 7).

These values, plotted against their respective discharges on logarithmic paper, using the method of Leopold and Maddock (1953), showed a straight-line relation for the channels of the White River (fig. 8). For such a straight line, the relation between the variables can be expressed in terms of the following equations:

$$\begin{aligned}w &= aQ^b \\ d &= cQ^f \\ v &= kQ^m\end{aligned}$$

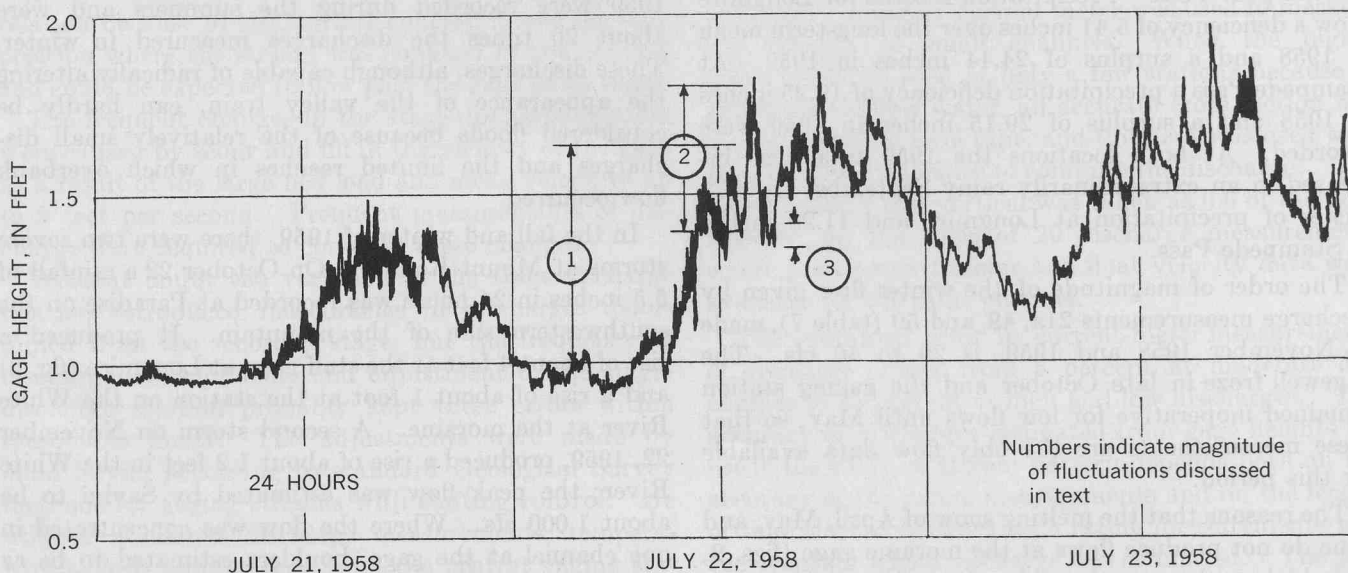


FIGURE 7.—Gage height record of the White River at moraine gage for July 21–23, 1958.



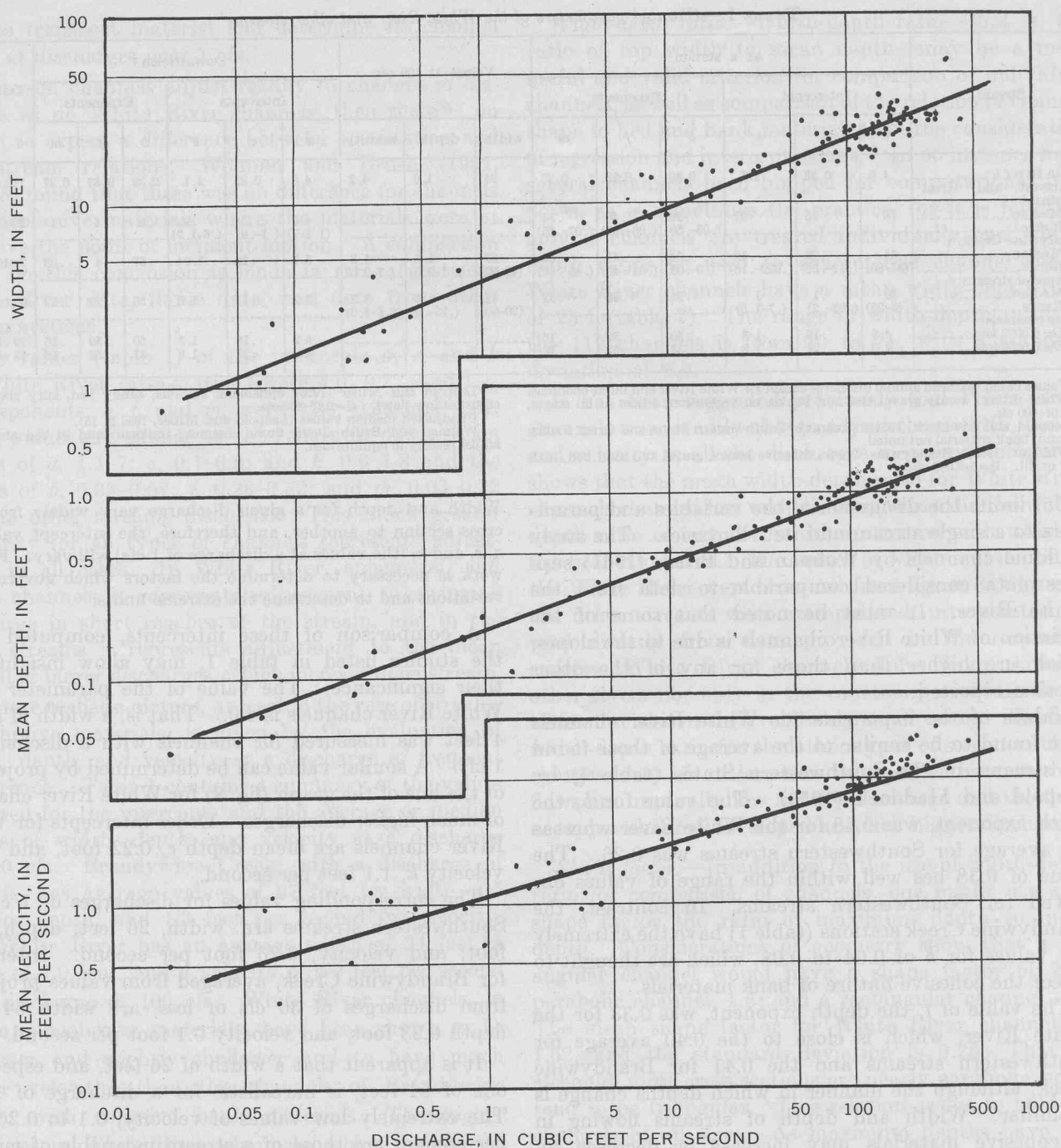


FIGURE 8.—Relation of mean velocity, mean depth, and width to discharge of the White River channels.

The exponents  $b$ ,  $f$ , and  $m$ , which represent the slopes of the respective lines, and the coefficients  $a$ ,  $c$ , and  $k$ , which represent the intercepts at a discharge point equal to 1 cfs, are parameters characteristic of a given situation and remain constant for that situation.

Values of the parameters determined for the White River, and those for other rivers for which values could be determined from the literature, are summarized in table 1. Changes in width, depth, and veloc-

ity in relation to discharge were compared using the values for changes "at-a-station," because the scatter caused by measuring in a number of different channels is thought to be analogous to that shown by Wolman (1955) to be caused by measuring at different points in the vicinity of a gaging station. The wide range of values in table 1, and in some cases lack of information about the parameters, is due to the great diversity of streams considered. Only one paper (Wolman,



TABLE 1.—Channel parameters of the White River and other streams

Stream	At a station						Values at 100 cfs			Downstream <sup>1</sup>						Data based on—
	Intercepts			Exponents						Intercepts			Exponents			
	<i>a</i>	<i>c</i>	<i>k</i>	<i>b</i>	<i>f</i>	<i>m</i>	width	depth	velocity	<i>a</i>	<i>c</i>	<i>k</i>	<i>b</i>	<i>f</i>	<i>m</i>	
White River <sup>2</sup>	4.0	0.22	1.1	0.38	0.33	0.27	24	1.0	4.2	4.0	0.22	1.1	0.38	0.33	0.27	112 channels.
Southwest and Great Plains: <sup>3</sup>																
Median.....	26	.15	.37	.26	.40	.34				4.0	.5	.9	.5	.4	.1	20 stations.
Range.....				(.03-.59)	(.06-.63)	(.07-.55)				(1.1-7)	(.1-.9)	(.6-1.2)				
Brandywine Creek: <sup>4</sup>																
Median.....	54	.23	.10	.04	.41	.55	67	1.5	1.3	5.7	.19	1.04	.57	.4	.03	7 stations.
Range.....	(37-80)	(.12-.52)	(.022-.16)	(.0-.08)	(.32-.46)	(.48-.69)	(45-90)	(1.1-2.0)	(.7-2.0)							
Ephemeral streams: <sup>5</sup>																
Median.....	10	.1	.1	.26	.33	.32	45	.5	3.3	5.2	.11	1.5	.50	.28	.22	9 stations.
Range.....	(3-26)	(.03-.2)	(.7-1.5)				(20-80)	(.25-.9)	(2.5-5.0)							
Flume channels: <sup>7</sup>																
$D_{50}$ 0.0022 foot.....	4.2	.18	1.3	.50	.39	.16				4.2	.18	1.3	.50	.39	.16	29 channels.
$D_{50}$ 0.0066 foot.....	2.0	.27	1.8	.33	.52	.16				2.0	.27	1.8	.33	.52	.16	13 channels.

<sup>1</sup> Values based on mean annual discharge except for White River and flume channels.

<sup>2</sup> White River: sandy gravel bed and banks, no vegetation, adjust at all stages,  $Q=0.01-400$  cfs.

<sup>3</sup> Leopold and Maddock (1953): streams of Southwestern States and Great Plains, bed and bank material not noted.

<sup>4</sup> Wolman (1955): Brandywine Creek, cohesive banks, gravel and sand bed, little scour or fill.  $Q=5-2,000$  cfs.

<sup>5</sup> Leopold and Miller (1956): ephemeral streams, sandy bed, silty clay banks, adjust at low flows.  $Q=0.15-800$  cfs.

<sup>6</sup> Unadjusted median values (Leopold and Miller, 1956 p. 15).

<sup>7</sup> Wolman and Brush (1961): flume channels, uniform sand in bed and banks, adjust readily to equilibrium.

1955) limits the discussion of the variables and parameters to a single stream and its tributaries. The study of flume channels by Wolman and Brush (1961) supplies data considered comparable to data from the White River. It must be noted that some of the variation of White River channels is due to the slopes, which are higher than those for any of the other streams reported.

Values of the exponents for White River channels were found to be similar to the average of those found for streams of the Southwestern States (table 1) by Leopold and Maddock (1953). The value for *b*, the width exponent, was 0.38 for the White River whereas the average for Southwestern streams was 0.26. The value of 0.38 lies well within the range of values observed for Southwestern streams. In contrast, the Brandywine Creek stations (table 1) have the extremely low values for *b* of 0.04 to 0.08, which are thought to reflect the cohesive nature of bank materials.

The value of *f*, the depth exponent, was 0.33 for the White River, which is close to the 0.40 average for Southwestern streams and the 0.41 for Brandywine Creek, although the manner in which depths change is dissimilar. Width and depth of streams flowing in noncohesive materials may increase or decrease by scour and deposition, or by changes in effective boundary roughness as well as by changes in discharge. Streams in cohesive bank materials change primarily by flowing at different depths in a channel determined by previous discharges.

The value of *m*, velocity exponent, for the White River was 0.27, which was not materially different from the 0.34 average of the Southwestern streams data but differed considerably from the value of 0.55 for Brandywine Creek.

According to Leopold and Maddock (1953, page 9),

Width and depth for a given discharge vary widely from one cross section to another, and therefore, the intercept values of *a*, *c*, and *k*, [the values at a discharge of 1 cfs] will vary. Further work is necessary to determine the factors which govern these variations and to determine the extreme limits.

A comparison of these intercepts, computed from the studies listed in table 1, may allow insight into their significance. The value of the parameter *a* for White River channels is 4.0. That is, a width of about 4 feet was measured for channels with a discharge of 1 cfs. A similar value can be determined by projection of the line of the graph (fig. 8) for White River channels of much higher discharges. Other intercepts for White River channels are mean depth *c*, 0.22 foot, and mean velocity *k*, 1.1 feet per second.

The corresponding values for discharges of 1 cfs for Southwestern streams are: width, 26 feet; depth, 0.15 foot; and velocity, 0.26 foot per second. Intercepts for Brandywine Creek, averaged from values projected from discharges of 30 cfs or less, are width 54 feet, depth 0.23 foot, and velocity 0.1 foot per second.

It is apparent that a width of 26 feet, and especially one of 54 feet, is unrealistic for a discharge of 1 cfs. The extremely low values of velocity, 0.1 to 0.26 foot per second, are those of a stream incapable of moving any of its bed or bank materials. The Brandywine Creek data would seem to indicate that the extreme values of width and velocity cannot be explained entirely by error introduced through extrapolation, since the extrapolation is not large, but reflect instead the failure of the channel to adjust to small discharges.

It is interesting to note that the intercept values for ephemeral streams (Leopold and Miller, 1958) 9 feet wide, 0.1 foot deep, with a velocity of 1 foot per second, show neither extreme widths nor extremely small velocities, but present a picture of a stream

able to transport material and determine its channel shape at discharges near 1 cfs.

If stream channels adjust readily to changes in discharge as do White River channels, then there is no reason to expect a difference between at-a-station and downstream relations. Wolman and Brush (1961, table 5) found that there was no difference for channels in noncohesive materials where the materials were at or above the point of incipient motion. A comparison based on this conclusion is made in table 1 between White River data, flume data, and data from other stream systems.

The values (table 1) of the intercepts  $a$ ,  $c$ , and  $k$  for White River (at-a-station) were 4.0, 0.22, and 1.1; the exponents,  $b$ ,  $f$ , and  $m$ , were 0.38, 0.33 and 0.27. These values are similar to the ranges in downstream values of  $a$ , 1.1–7;  $c$ , 0.1–0.9; and  $k$ , 0.6–1.8 and the ranges of  $b$ , 0.33–0.57;  $f$ , 0.28–0.52; and  $m$ , 0.03–0.22 for the other streams discussed. This much greater similarity for all types of channels represents adjustment in all cases. In White River, ephemeral, and flume channels it represents adjustment to changing discharge in short reaches of the stream, and in the other streams it represents adjustment to the mean annual or larger discharges, which increase downstream.

A more realistic picture, at least in the case of streams in cohesive materials, is given by the evaluation of width, depth, and velocity at a discharge of frequent occurrence. Table 1 contains a summary of the values obtained for the intercepts and the average or median values of width, depth, and velocity at a discharge of 100 cfs. Brandywine Creek, with a discharge of 100 cfs, has average values of 67 feet for width, 1.5 feet for depth, and 1.3 feet per second for velocity. The White River has an average width of 24 feet, a depth of 1.0 foot, and a velocity of 4.2 feet per second at a discharge of 100 cfs. White River channels, in coarse noncohesive materials, have been shown to be narrower and slightly shallower and to have much higher velocities than the channels of Brandywine Creek, with cohesive bank materials. The higher velocities in White River channels, which are due to the high slope, permit smaller cross-sectional areas to carry the same flows.

#### SHAPE

Individual elements of the cross-sectional shape of White River channels have been described in the preceding section in relation to the flow that produced that shape. The shape of the channel cross sections now will be illustrated and the elements of shape compared by considering the width-depth ratio and a shape factor.

*Width-depth ratio.*—Width-depth ratio—that is, the ratio of top width to mean depth—may be a more useful and valid criterion for comparison of individual channels, as well as comparison of the relation of channel shape to bed and bank material, than the consideration of regression and intercepts alone. In no instance have several channels been lumped for computational purposes, as is sometimes the practice, for it is felt that only if channels are treated individually can width-depth ratios be used to characterize channel shape. White River channels have a mean width-depth ratio of 23.1 (table 7). The range of width-depth ratios of the 112 channels is from 10 to 71, with a standard deviation of 9.9.

It has been suggested previously that White River channels were readily able to adjust their width, depth, and velocity at all the discharges measured. Figure 9 shows that the mean width-depth ratio for White River channels changed less with discharge than did the width-depth ratio of any of the other streams with which it has been compared. The values of width and depth for the White River were obtained from figure 8; those for the other streams were extrapolated, interpolated, or read directly from data provided in the respective studies. White River channels differ from those of other rivers not only in the constancy of width-depth ratios but also in their relatively small value. The high width-depth ratio for the other types of streams at discharges of 10 cfs or less is to be expected because these flows are unusually low for these streams and the flows occupy a channel developed at much higher discharges.

*Shape factor.*—In addition to the use of width-depth ratio for comparison of channels, one might use as a shape factor the ratio of maximum depth to mean depth. Considerations of geometry show that a triangular channel would have a shape factor of 2; a parabolic channel, 1.5; and a rectangular channel, 1.0. The mean shape factor for White River channels is 1.62, and the standard deviation is 0.26. That is, although most cross sections are nearly parabolic, some tend to be triangular. This relation seems to hold for channels of all sizes. As maximum depths have not been reported in the other publications listed in table 1, a comparison with other types of streams cannot be made. Only with further use can the value of this shape factor as an indicator of channel shape be estimated. It must be recognized, however, that a shape factor of 1.5 is a necessary, but not sufficient, condition to prove that a channel is parabolic in cross section. Although it is possible that a variety of weird cross sections could have a shape factor of 1.5, it is probable that a channel in alluvial materials that has such a shape factor is nearly parabolic.

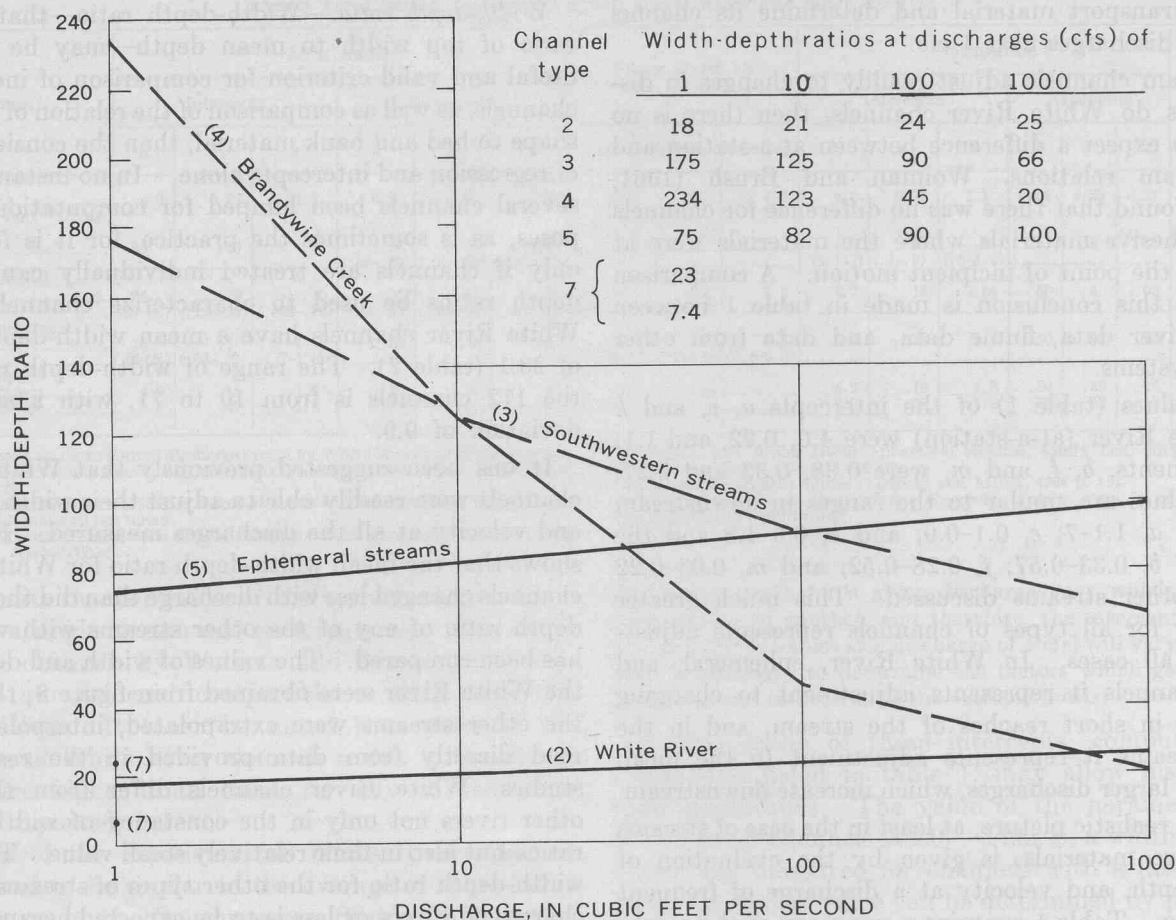


FIGURE 9.—Relation of average width-depth ratios to discharge. Channel type numbers refer to table 1 footnotes, where channels are described.

Figure 10 illustrates the manner in which shape factor and width-depth ratio represent the shape of some White River channels. Neither width-depth ratio nor shape factor indicates the irregularity of the cross section.

#### MODE OF CHANGE

The rapid adjustment to increasing flow and therefore increasing bed load in non-cohesive materials appears to take place by increasing width and depth in the following manner. The initial channel widens and may even grow shallower, until at some point the critical tractive force is less than that necessary for transport of the bed load. The material deposited causes further widening and a decrease in the tractive force. Deposition or cutting of adjacent channels eventually brings the bar above water, resulting in the development of two channels similar in shape to the original channel but adjusted to the higher flow condition. Additional flow repeats the sequence of events in either or both of the new channels.

Adjustment of the White River channel shape at low discharges takes place by both erosion and deposition.

With decreasing discharge, the adjustment might take place as follows:

1. The bed load decreases sharply.
2. The velocities, especially near the bank, are lower due to presence of shallower water at the sides of the channels and occasional boulders in the channel near the bank. These boulders remain after being undermined and dislodged from the bank when the surrounding finer materials were removed by velocities incapable of moving the boulders. As a result of these lower velocities, deposition of the coarser fraction of the suspended load takes place at the sides of the channel.
3. At the same time, velocities remain high enough to permit scour in the center part of the channel as the depth decreases with the decreasing discharge. The result is that similar channel shapes are possible for widely different discharges, whether increasing or decreasing.

Wolman and Brush (1961) noted that in flume channels, at a constant slope and with a constant introduction of load at the same rate that material was being



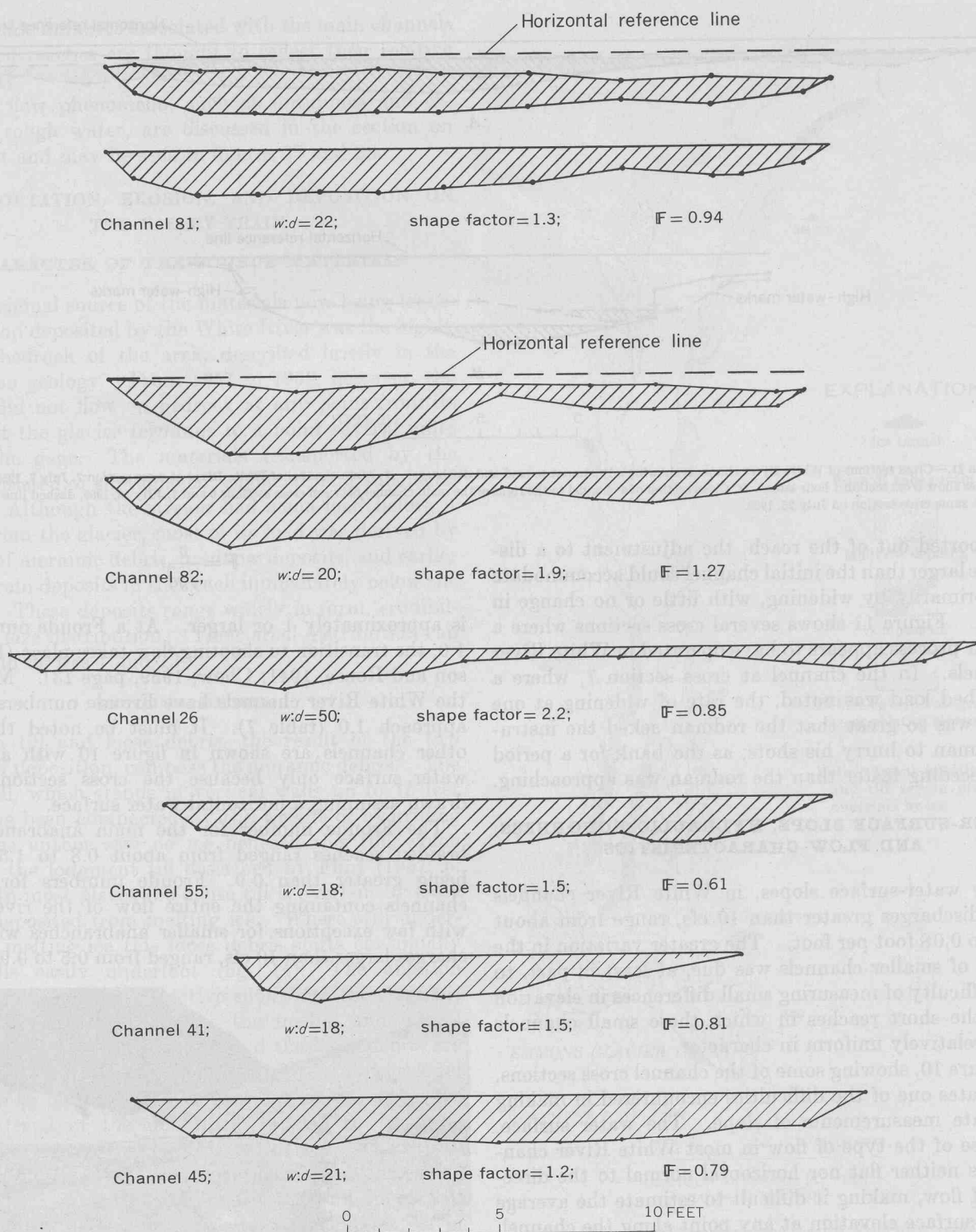


FIGURE 10.—Cross sections of several White River channels showing width-depth ratio, shape factor, and Froude number. Water surface assumed level except in upper cross sections of channels 81 and 82; maximum difference in water surface elevation: channel 81, 0.6 foot; channel 82, 0.6 foot.

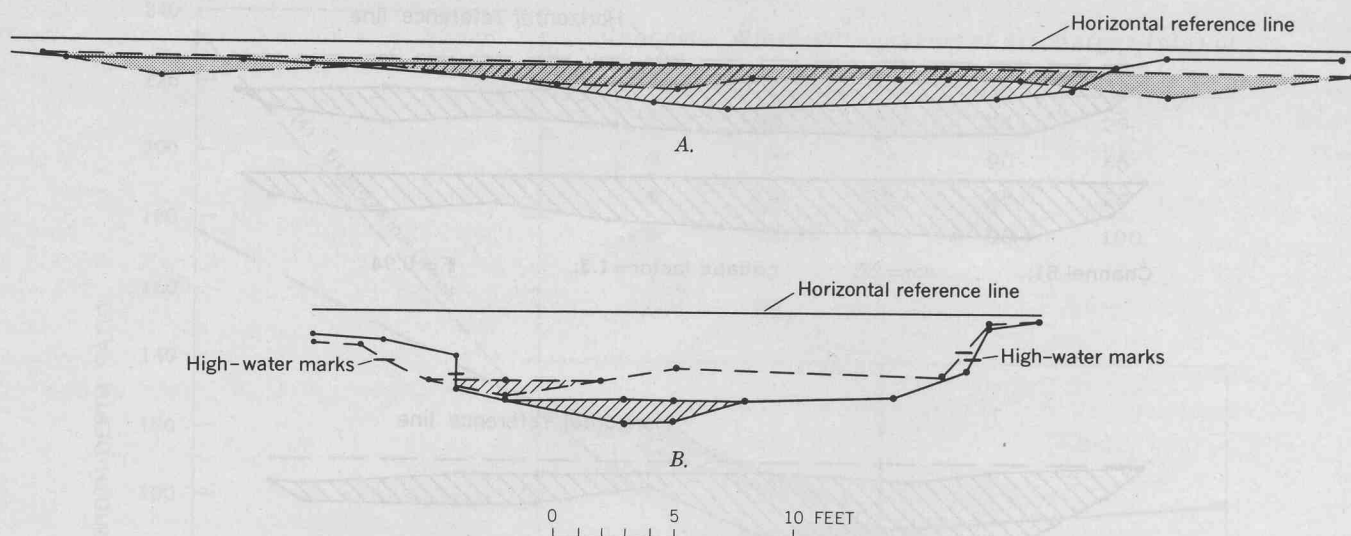


FIGURE 11.—Cross sections of White River channels showing adjustment to load and discharge. A, Main channel of White River at cross section 7, July 7, 1958; dashed lines show cross section 1 hour later. B, Cross section of a channel to the right of the main channel between cross sections 6 and 7, July 16, 1959; dashed line indicates the same cross section on July 25, 1959.

transported out of the reach, the adjustment to a discharge larger than the initial channel could accommodate was primarily by widening, with little or no change in depth. Figure 11 shows several cross sections where a similar process appears to have operated in White River channels. In the channel at cross section 7, where a large bed load was noted, the rate of widening at one point was so great that the rodman asked the instrument man to hurry his shots, as the bank for a period was receding faster than the rodman was approaching.

#### WATER-SURFACE SLOPE, HYDRAULIC ROUGHNESS, AND FLOW CHARACTERISTICS

The water-surface slopes, in White River channels with discharges greater than 10 cfs, range from about 0.01 to 0.08 foot per foot. The greater variation in the slopes of smaller channels was due, at least in part, to the difficulty of measuring small differences in elevation over the short reaches in which these small channels were relatively uniform in character.

Figure 10, showing some of the channel cross sections, illustrates one of the difficulties encountered in making accurate measurements of slope. The water surface, because of the type of flow in most White River channels, is neither flat nor horizontal normal to the direction of flow, making it difficult to estimate the average water-surface elevation at any point along the channel. Two estimates of elevation were included in every measurement of water-surface slope.

The uneven water surfaces shown in figure 12 and channels 81 and 82, figure 10, are characteristic of channels in which the Froude number,

$$F = \frac{v}{\sqrt{gd}}$$

is approximately 1 or larger. At a Froude number of 1.0, the transition to shooting flow takes place (Robertson and Rouse, 1941; Chow, 1959, page 13). Many of the White River channels have Froude numbers which approach 1.0 (table 7). It must be noted that the other channels are shown in figure 10 with an even water surface only because the cross sections were drawn assuming a horizontal water surface.

The Froude number for the main anabranches of braided reaches ranged from about 0.8 to 1.5, most being greater than 0.9. Froude numbers for single channels containing the entire flow of the river, and with few exceptions for smaller anabranches with discharges larger than 10 cfs, ranged from 0.5 to 0.9. The

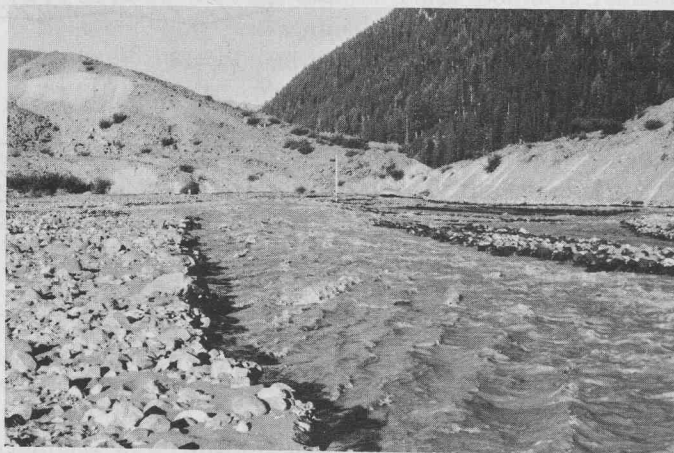


FIGURE 12.—Standing waves in a White River channel.

high Froude numbers associated with the main channels of braided reaches are thought to reflect their relative instability.

Other flow phenomena, such as antidunes and extremely rough water, are discussed in the section on transport and may be seen in figures 27 and 28.

#### TRANSPORTATION, EROSION, AND DEPOSITION ON THE VALLEY TRAIN

##### CHARACTER OF THE SOURCE MATERIALS

The original source of the materials now being transported and deposited by the White River was the highly diverse bedrock of the area, described briefly in the section on geology. From 1957 to 1959, however, the stream did not flow on bedrock at any point from its source at the glacier terminus to a point several miles below the gage. The materials transported by the river consisted of supraglacial, englacial, and subglacial debris. Although the stream had some load before it issued from the glacier, most of its load was derived by erosion of morainic debris, mudflow deposits, and earlier valley-train deposits in the reach immediately below the glacier. These deposits range widely in form, erodibility, and size distribution. Their areal distribution can be seen in figures 1 and 13.

##### MORAINIC DEBRIS

Dense till and loose debris, the result of the two modes of deposition, compose the morainic debris. The dense till, which stands in vertical walls up to 15 feet high, has been compacted by the weight of overriding ice. The author saw no ice beneath this till, which may be the lodgment till described by Flint (1957, p. 120). In most places the dense till is overlain by loose debris deposited from melting ice. Where still underlain by melting ice this loose debris shifts continually and rolls easily underfoot (fig. 14). The ablation debris mantles both the active snout and the stagnant ice of the glacier and forms the medial and lateral moraines. Both the dense till and the loose debris are composed of materials from clay size to occasional boulders as large as 10 feet in intermediate diameter, but material of the dense till appears to be more rounded than the angular ablation debris. The supraglacial ablation deposits are characterized by irregular topography. Locally they appear to have been sorted. Occasionally material of one size predominates, as in some pockets of coarse boulders that contain no fine materials; apparently continual shifting due to the melting of underlying ice has "sifted out" the fines. Walking in such areas is treacherous, owing to the delicate balance of some of the large boulders and the large spaces between them. Within this loose morainic

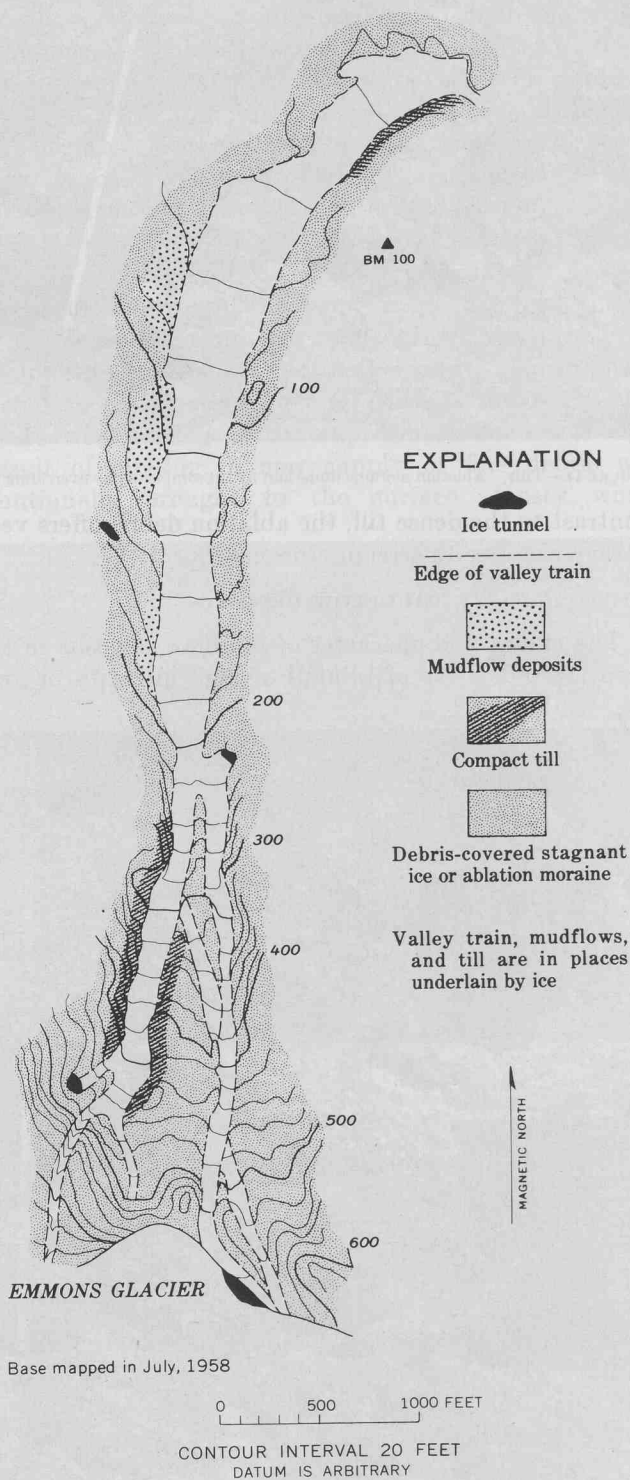


FIGURE 13.—Distribution of deposits on and near the White River valley train.

debris there are occasional small patches of bedded deposits. Ablation debris stands at what must be its normal angle of repose in banks cut by the stream. In





FIGURE 14.—Tills. Ablation moraine underlain by till compacted by overriding ice.

contrast to the dense till, the ablation debris offers very little resistance to stream erosion.

#### MUDFLOW DEPOSITS

The extent and character of mudflow deposits in the area are indicative of rainfall of high intensity or long

duration, during which the steep faces of stagnant ice were washed clean and large amounts of morainic debris fell, slumped, or flowed from the ice. This movement was evidently concentrated in amphitheatrelike basins, from which the debris-laden mud flowed onto moraine and valley-train deposits, forming fans that were often truncated by stream erosion. Minor mudflows also occurred during periods of intense melting.

Mudflow deposits (figs. 15–19) are dense and unsorted and usually occur in fans composed of a series of flows. The top part of each flow is sorted material usually of sand size. Levees and boulder fronts (figs. 18, 19, and 38) are surficial features similar to those described by Sharp (1942), and Sharp and Nobles, (1953). The levees are depositional features thought to have formed when coarse debris at the sides of the flow lost mobility while material in the center of the flow continued. The height of the levees is usually emphasized by subsequent erosion by small streams. The boulder fronts are similar to the levees; they were apparently formed when coarse materials lost mobility and formed a dam



FIGURE 15.—Cutbank in mudflow sequence. Note stream deposits at the top of each mudflow layer. The noncohesive materials in the stream deposits have fallen from between the more cohesive clayey layers.

in the path of flow, stopping the flow or diverting part of it.

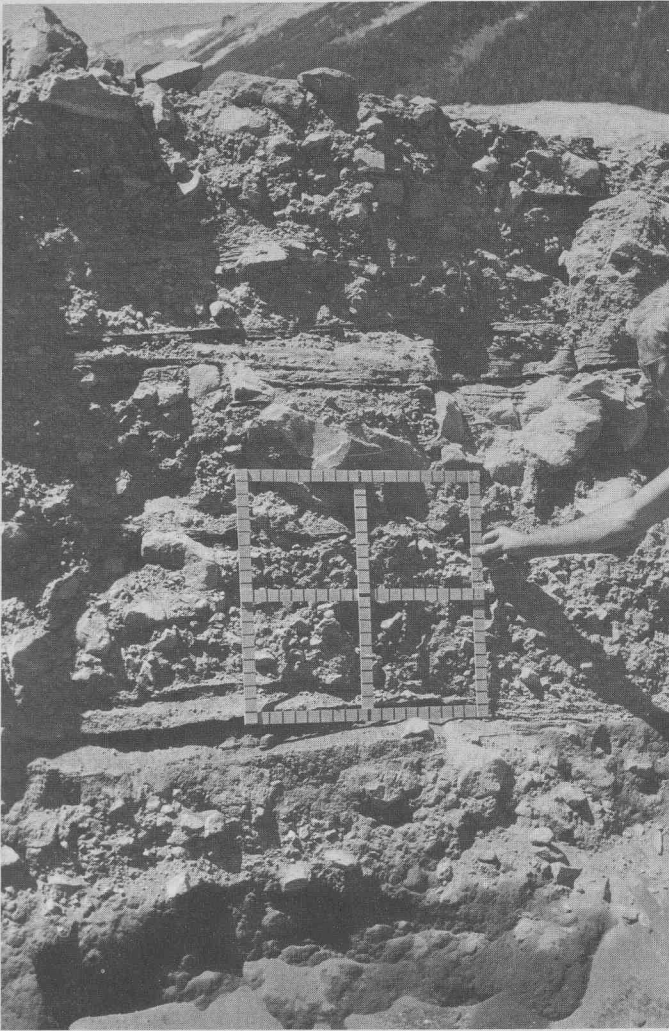


FIGURE 16.—Detail of cutbank in mudflow sequence.



FIGURE 17.—Mudflow deposit. This material stands although undercut as much as 3 feet.

Most sizes of debris apparently were present in these mudflow deposits. Boulders 2 to 3 feet in maximum diameter were moved by mudflows about 2 feet deep. In the course of the fieldwork, a bank of mudflow material 3 feet high, undercut as much as 3 feet by the stream, was observed to support the weight of a person jumping on it, probably because of the presence of a large percent of clay-size material (fig. 17), whereas the moraine and valley-train deposits were observed not to withstand more than 6 inches to a foot of undercutting without collapsing under their own weight.

In places the wind had moved the finer fraction of stabilized mudflow and morainic deposits and produced deflation pavements and small drifts or dunes. Where there was shifting of the supraglacial materials as a result of melting, a new supply of fine debris was continually brought to the surface. Gusty winds



FIGURE 18.—Mudflow truncated in foreground by stream. Note ice faces in wall of amphitheater in background, coarse material in levees, boulder fronts on surface of mudflow to the right rear of man.



FIGURE 19.—Boulder front on surface of mudflow shown in figures 15 and 16.



reworked these materials and produced numerous "dust devils" and occasional dust clouds.

#### VALLEY-TRAIN DEPOSITS

Valley-train materials provide a source of load to the stream throughout its length. Much of the material deposited downstream from the junction of East and West Emmons Creeks had previously been eroded from valley-train deposits upstream. Because such deposits are also a source of material, they are discussed here in relation to the other source materials, although the size of the materials in the valley train is discussed in detail in the section on pebble counts.

The valley-train materials are as large as 3 feet in diameter and are angular to subrounded. The fine gravel and smaller sizes show little rounding. Bedding can usually not be traced for more than a few feet (figs. 20, 21). Sand carried by the stream fills most interstices between the coarser particles as soon as the coarse particles come to rest. In places coarse particles appear to be widely scattered at the surface because the most recent streams on the surface were small ones which deposited fine materials, burying the coarser deposits of earlier streams (fig. 22). Even with such burial, coarse materials invariably occur close to the surface if not on the surface at any point.

Hjulström (1935, p. 326-327) suggested that empirical data show that a 10-cm-diameter particle can be transported by a stream without erosion over a bed of silt and clay or over a bed of material 3 to 4 cm in diameter and larger, but not over material ranging from 0.0001 to 4 cm. A 5-cm particle could be transported over silt and clay, and materials 2 cm and larger, but not over material from 0.0001 to 2 cm. He concluded that

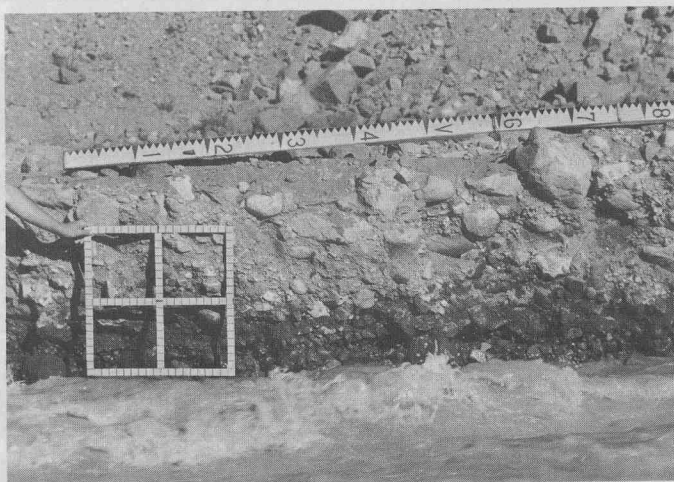


FIGURE 20.—Cutbank in valley train. Note concentration of fine debris at surface. Vertical bank is due to stream erosion of materials with apparent cohesion.



FIGURE 21.—Cutbank in valley train. Note poised boulders, a possible lag concentration at surface. Mixing of sizes is apparent in the top of the sandy layer half way up the bank. Bedding is limited in its horizontal extent.

it would be unusual to find such coarser materials on top of the finer ones noted and suggested:

Should this be the case, two alternatives appear:

1. The coarser material was not brought there by running water. In this case there may be a clearly defined contact surface.
2. The coarser material was brought there by a stream which eroded away the finer material, but had no time to erode it away entirely before the velocity of the water diminished and deposition occurred. There is a border zone where the two kinds of material are mixed.

Evidence of the correctness of Hjulström's conclusions is offered by figures 15-22. Figures 15-19 show mudflow deposits from the stagnant ice adjacent to the valley train. Figure 15 shows large boulders deposited immediately above undisturbed thinly bedded sands, which could not have been done by running water. Figure 20, a typical valley-train deposit, shows concentration of fine materials only at the top, although fines are mixed throughout the rest of the deposit. Figure 21 shows the mixed border zones mentioned in alternative 2 above.

The nearly vertical cutbanks of valley-train deposits, to 10 feet in height, demonstrate an apparent cohesion yet may give way when a person steps close to the edge. After drying, the banks lose this ability to stand vertically and assume an angle of repose. Drying rarely occurs, however; the gravel is usually damp within a few inches of the surface. After a few humid cloudy days without precipitation, the surface of the valley train becomes darkened by moisture thought to be the result of decreased evaporation at the surface.





FIGURE 22.—Small stream depositing fines which bury coarser deposits. Note antidunes.

At times, drops of water are actually seen on the surface of the material. It is thought that this moisture is either brought to the surface by capillary action or results from condensation. Deposits of a white substance which tastes somewhat like sodium bicarbonate form on rocks lying on the wettest areas of the valley train, suggesting that capillary action is present.

Cohesion due to the moisture in the deposits appears to be weakened by the addition of more water. Bank erosion is thus due, at least in part, to the saturation of a "cohesive" bank by the wash of a wave; material is left overhanging until it caves. The rate of bank erosion appears to depend on the saturation of vertical banks as well as the vigor of attack by the current.

#### ANALYSES OF PARTICLE SIZE

Size analyses of the three types of materials help to explain the differences in appearance, erodibility, and stability. The results, while based on a few samples, suggest that analysis of the finer sizes could be quite helpful in determining the mode of deposition of materials of questionable origin.

#### SIEVE ANALYSES

With the exception of the valley-train materials, two sieve analyses of the finer fraction (less than 53 mm or 2.1 inches) of each of the three types of source material showed no consistent differences. The valley-train deposits were deficient in the smaller sizes. Analyses by Richard Arnold (written communication) of the fraction less than 2 mm in size of one sample of the valley train and one of the mudflow, using a combined sieve analysis and bottom-withdrawal tube method, led him to conclude that the stream deposit showed a washing out of silt and clay size materials. He found that 40 percent of the mudflow material was silt and clay (less than 0.105 mm) as opposed to 10 percent of the valley-train material.

These results help to explain why dry moraine and mudflow materials when disturbed are much dustier than the valley-train materials. In addition, small clear streams emerge at various points on the valley train—some from kettles developed in the valley train and others from springs in abandoned channels—

whereas small streams from ice faces and mudflow areas are usually very muddy.

#### PEBBLE COUNTS

Size changes with distance from the glacier were observed in the valley-train materials. To measure these changes by sieve analysis was impractical because of the large samples that would have been required because of the coarseness of the materials. Pebble counting was adopted as the most rapid and accurate method of measurement. By means of pebble counts size classifications were made of randomly selected particles from the surface of the deposits, in order to develop size-distribution curves. For a detailed discussion of the technique used, see Wolman (1954).

Pebble counting demonstrated a systematic decrease in median diameter of the valley-train materials with distance from the source. Sampling at each cross sec-

tion was limited to the area that appeared to have the coarsest materials at the surface, because very small streams, present almost everywhere on the valley train at some time during the summer, tended to bury the coarser deposits thought to be more characteristic of the load of the main channel (fig. 22). The sample sites were selected near the center of the valley train in order to minimize the effects of local bank erosion of coarser materials. Sizes less than 4 mm in diameter were not differentiated, and this size class is shown in table 2 only to indicate the percentage of area covered by finer material. Comparisons are, thus, based on the fraction greater than 4 mm in diameter.

The range in median diameter of samples was from 180 mm to 120 mm (0.59 to 0.39 foot) in table 2. A median diameter of 180 mm was found for material sampled near cross section 2 on a valley slope of 0.11 foot per foot, 2,350 feet below the glacier, whereas a

TABLE 2.—Size distribution of valley-train materials

[Percentages and  $D_{50}$ ,  $D_{25}$ , and  $D_{75}$  are based on samples more than 4 mm in size]

Miles from source	Valley slope (ft/ft)	Percent in size range (mm)											D <sub>50</sub> (mm)	D <sub>25</sub> (mm)	D <sub>75</sub> (mm)	Location
		From To	4 8	8 16	16 32	32 64	64 128	128 256	256 512	512 1024	1024+	<4				
White River between Emmons Glacier and moraine																
0	0.20	-----	0	0	8	16	16	25	29	6	0	4	180	71	300	Upper West Emmons Creek.
.36	.20	-----	0	2	0	13	20	36	22	4	2	8	177	95	270	Lower West Emmons Creek.
.43	.11	-----	1	0	3	7	21	39	24	5	0	8	180	106	270	Cross section 2.
.64	.075	-----	1	2	6	12	20	29	28	2	0	11	175	77	270	Cross section 3.
.83	.05	-----	2	3	8	10	15	37	25	0	0	10	165	71	255	Cross section 5.
.95	.04	-----	1	3	4	7	25	47	13	0	0	5	145	94	200	Cross section 6.
1.12	.03	-----	1	4	14	18	14	38	11	0	0	43	125	40	188	Cross section 10.
1.23	.03	-----	0	1	3	16	36	39	5	0	0	3	120	73	160	Cross section 12.
White River below moraine																
1.7	0.066	-----	0	0	1	4	12	47	33	3	0	3	210	150	320	Just below moraine.
2.2	.055	-----	1	3	5	5	27	38	19	2	0	10	150	94	230	Cross section 13.
4.5	.018	-----	0	0	2	9	37	36	16	0	0	4	130	84	215	Near park dump.
14.8	.018	-----	0	1	3	15	55	26	0	0	0	21	105	73	130	Cross section 14.
24.8	.027	-----	0	0	0	5	52	43	0	0	0	3	115	83	155	Near lumber company gate.
28.4	.0085	-----	0	1	2	10	29	55	3	0	0	25	143	94	185	Below West Fork River junction.
West Fork White River below Winthrop Glacier																
7.5	0.024	-----	1	0	5	20	31	41	2	0	0	4	110	62	165	Below end of road.
14.0	.020	-----	0	0	1	23	54	21	1	0	0	2	89	64	125	At logging bridge.
Carbon River below Carbon Glacier																
0.9	0.058	-----	1	0	1	8	11	34	41	4	0	2	230	145	350	½ mile above end of road.
3.5	.034	-----	0	6	8	5	18	39	23	1	0	10	160	91	250	Ipsut Creek campground.
9.0	.015	-----	0	1	8	17	42	27	5	0	0	4	94	62	145	Just outside park.
Nisqually River below Nisqually Glacier																
0	0.128	-----	2	3	5	5	8	20	25	24	8	17	310	140	630	Below stagnant ice.
.5	.094	-----	3	3	3	3	8	16	38	22	4	11	325	170	520	Above Glacier Bridge.
2.8	.057	-----	7	4	4	11	19	20	21	13	1	17	153	61	360	Above Power Plant Bridge.
4.3	.040	-----	2	4	4	4	9	30	39	8	0	27	240	130	380	Longmire campground.

median diameter of 120 mm was recorded for materials at cross section 12 on a slope of 0.03 foot per foot, 6,600 feet from the glacier. The change of 60 mm in median diameter for the valley-train materials over a distance of some 4,200 feet can hardly be explained by diminution of the materials by wear. Stream materials batter each other, but a one-third reduction in median diameter by this means would seem quite unlikely, and this change would strongly suggest that selective transport must have produced radical changes in size.

In general, the median diameter of the valley-train materials decreases with distance below the glacier (fig. 23). Increases in median diameters such as those at pebble count sites 1.7, 25, and 28 miles downstream from the glacier call for a logical explanation. The major increase at the 1.7-mile site can be readily accounted for by the presence of a new source of extremely coarse material in the massive valley-loop moraine through which the stream flows 1.2 miles below the glacier. An explanation for the size increase at the 25-mile locality developed when D. R. Crandell (oral communication) reported a previously unknown moraine about 17 miles downstream from the Emmons Glacier. This suggests that size analysis of valley-train materials may offer clues to the presence of moraines. Time did not permit investigation of the cause of the

even greater increase in size between the 25- and 28-mile sampling sites on the White River or the cause of the increase in median diameter at the 4.3-mile locality on the Nisqually River.

The relation between median diameter of material and valley slope shown in figure 24 demonstrates that slope decreases very rapidly with decreasing median diameter.

Both the Carbon and the Nisqually Rivers have histories of frequent and destructive floods. On the Nisqually River the average intermediate diameter of the 10 largest boulders deposited on and near the Glacier Bridge parking lot by the 1955 flood was 9 feet; one maximum diameter was 24 feet. Such extremely coarse deposits were avoided in sampling, but it is apparent from the scatter of points for the Nisqually River that in selecting pebble-count sites some of these deposits were included. A much more extensive and systematic sampling would be necessary to characterize these streams.

The relation between transportation, declivity, and discharge discussed at length by Gilbert (1877) has been summarized by Lane (1955b, p. 6) in the form of the proportionality.

$$Q_s d \propto Q_w^s$$

where  $Q_s$  and  $Q_w$  are quantity of sediment and water,

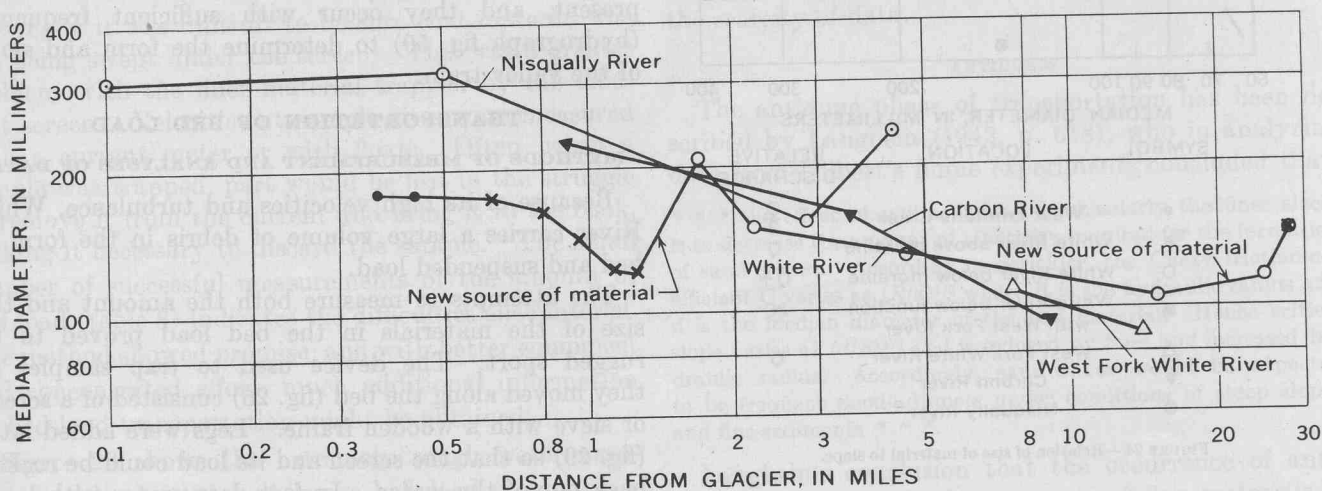


FIGURE 23.—Relation of size of material to distance below glacier.



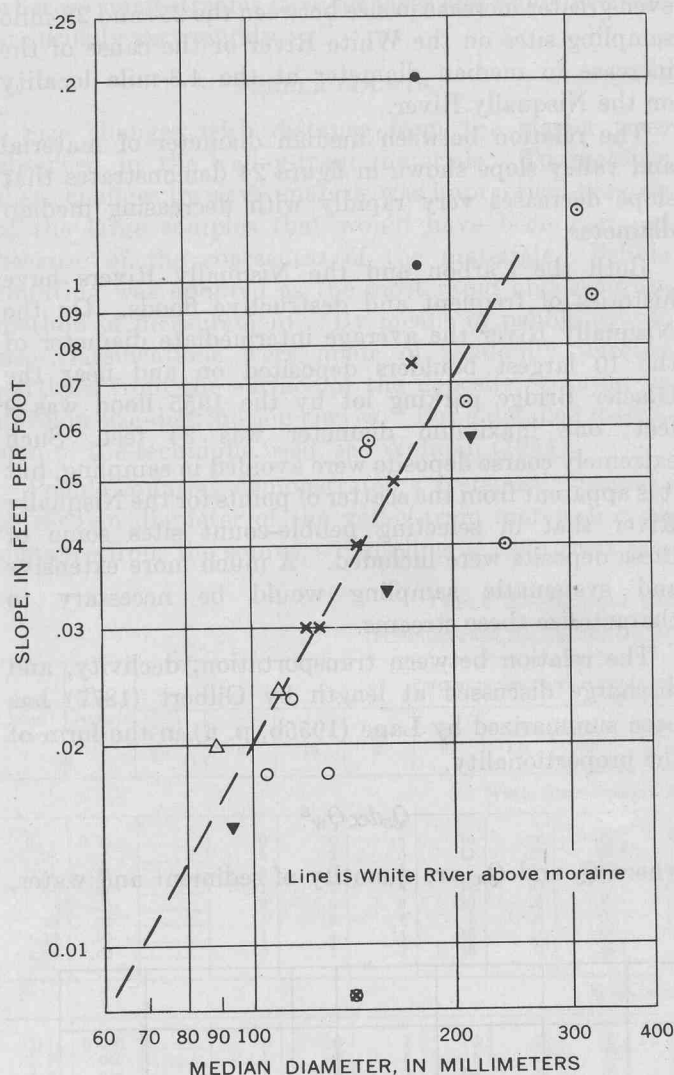


FIGURE 24.—Relation of size of material to slope.

respectively,  $d$  is particle diameter or size of sediment, and  $s$  is the slope of the stream.

At any given time, at all points in the reach of the White River between the junction of East and West Emmons Creeks and the gage, it can be assumed that the discharge ( $Q$ ) is approximately the same, because the contribution from the stagnant ice is small. From the proportionality above, a change in slope should

bring about a corresponding change in size of sediment or quantity of sediment or both. Measurements of erosion and deposition (figs. 36, 37, tables 4 and 5) revealed no location or slope at which there was always erosion or always deposition; thus, it is necessary to look for a change in particle size to keep the proportionality balanced. Figure 24 shows that for the White River in the reach above the moraine the slope varied approximately as the square of the particle size.

The slope of the White River (with its greater discharge,  $2Q$ ) below the mouth of the West Fork White River is far less than would be predicted from the relation between slope and intermediate diameter for the smaller discharge of the White River above the moraine. The values of slope for West Emmons Creek ( $\frac{Q}{2}$ ) above its confluence with East Emmons Creek are higher than for an area with similar-sized material and higher discharge below the junction.

If one accepts the hypothesis of Wolman and Miller (1960) that the dominant process is one that occurs with sufficient frequency and magnitude to cause most of the changes observed and is neither the frequent event, which is less than the threshold value, nor the infrequent catastrophic event, the slope-forming discharge lies between 200 and 500 cfs. These discharges are clearly capable of modifying not only the channels in which they flow but of changing the elevation of the valley train and transporting all sizes of materials present, and they occur with sufficient frequency (hydrograph fig. 50) to determine the form and slope of the valley train.

#### TRANSPORTATION OF BED LOAD

##### METHODS OF MEASUREMENT AND ANALYSES OF DATA

Because of its high velocities and turbulence, White River carries a large volume of debris in the form of bed and suspended load.

The attempts to measure both the amount and the size of the materials in the bed load proved to be rugged sport. The device used to trap samples as they moved along the bed (fig. 25) consisted of a screen or sieve with a wooden frame. Legs were added later (fig. 29) so that the screen and its load could be rocked back out of the water. In fast deep water with large quantities of materials in motion, a coarse screen with 0.175-foot (53-mm<sup>2</sup>) openings was used because it was easier to handle in the current and did not immediately become clogged with the smaller size fraction of the bed load. With lower velocities and depths and smaller quantities of material, a finer screen with 0.05-foot (15-mm) openings was used. Where the bottom

<sup>2</sup> This size was an expedient determined by the coarsest chicken wire that could be purchased in the nearest town.



FIGURE 25.—Sampling bed load with screen. Sample is from the antidune phase of transportation.

was rocky it was difficult to be sure that material was not being swept under the screen. This was a greater problem with the finer material trapped by the 0.05-foot screen. Velocities at sample sites were measured with a current meter or with floats. Often, when a sample was trapped, part would be lost in the struggle to remove it from the current and bring it to the bank, making it necessary to discard the sample. The small number of successful measurements of the amount of bed load (table 8) indicates the difficulties encountered. The method showed promise, and with better equipment and concentrated effort, much additional information on bed-load transportation might be obtained.

Figure 26 shows the extremely rough water associated with quantities of coarse bed load moving in a shallow channel with rough bottom. This is thought to be shooting or supercritical flow, for it shoots into the air over the top of obstacles instead of flowing around them. The boulders battering each other in such a reach make a terrific din and shake the ground for as much as 15 feet back from the bank. This shaking occasionally dislodges material poised on the banks. If one overcomes his awe of the noise, these rough,



FIGURE 26.—Large load moving over rough bed in shallow channel. Note flow over boulders.

shallow reaches can be waded more easily than those that are quieter, but deeper and swifter.

During the measurements of amount of bed load, its movement was observed to be discontinuous both in position in the channel and in time. Only at the highest discharges were large quantities of material in motion. Occasionally the materials could be traced to nearby bank erosion, but usually the source of the material was not readily apparent. With the exception of one set of antidune ripples, no conclusions as to the amount of bed-load movement were drawn because of the scarcity of data.

#### ANTIDUNES

The antidune phase of transportation has been described by Langbein (1942, p. 618), who in analyzing data from Gilbert's flume experiments concluded that:

\*\*\* the effect of sediment-load (particularly the finer sizes) is to decrease the amount of kineticity required for the formation of sand waves. According to Strickler, the Chezy friction-coefficient  $C$  varies as  $(R/d)^{1/6}$  where  $R$  is the hydraulic radius and  $d$  is the median diameter of the bed material. Hence critical slope varies as  $(d/R)^{1/3}$  and is reduced by fines and increased hydraulic radius. Accordingly, sand waves would be expected to be frequent flood-adjuncts under conditions of steep slopes and fine sediments \*\*\*.

Langbein's conclusion that the occurrence of antidunes is favored by the presence of fine materials is borne out by the observations along White River. Gilbert (1914, p. 32) noted however "Where the debris is very coarse, as in the outwash plains of glaciers, a din of clashing boulders is added to the roar of the water." Although in transport phases other than antidune a similar din was heard on the White River (fig. 26), the boulders transported in the antidune phase were rolling silently on a bed of sand.

Antidunes were observed when discharge increased markedly in a channel with predominantly sandy bed materials. Measurements of velocity, depth, slope, wavelength, size, and amount of the coarse fraction of bed load were made for the set of antidunes shown in figures 25 and 27-29 (table 8). Combing of the crests, mentioned by both Pierce (1917) and Gilbert (1914), is shown by the downstream dunes (fig. 27).

The average of three float velocities taken over a 100-foot reach exhibiting antidunes was 9.4 feet per second; a mean velocity of about 8.2 feet per second. Depths for troughs were found to differ consistently with those for crests. The average of 23 depth measurements made in troughs was 0.9 foot, the range from 0.6 to 1.0 foot. The average depth of 22 crest measurements was 1.2 feet, the range from 0.9 to 1.6

feet. Using a depth of 1 foot as the average over the reach in which the velocity was measured, the Froude number equals 1.45, clearly in the range of supercritical flow. The difference in depth between trough and crest is thought to result from: (a) a piling up of the water due to convergence of flow in the plan view, at the crests, and (b) the increase in depth over a rise in the channel bottom that occurs with supercritical flow (Rouse, 1946). The slope, measured using a steel tape and hand level, was 0.012 foot per foot—lower than average for this valley train. For other evidence of the nature of the flow see the piling up of the water against the observer in figure 28. Amplitudes of the dunes observed were as much as 1 foot; wavelengths averaged about 8 feet.



FIGURE 27.—Antidunes showing combing of crests on downstream dunes. Flow is from left foreground to right background.



FIGURE 28.—Measurement of amplitude and wavelength of antidunes. Yardsticks used for scale.

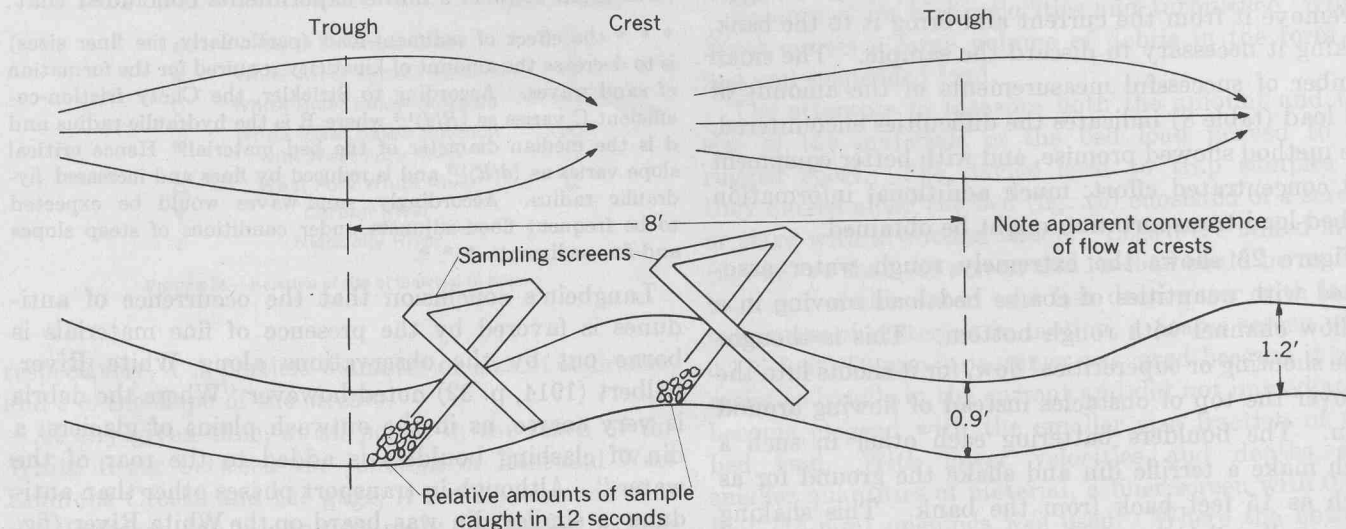


FIGURE 29.—Longitudinal cross section and plan view of antidunes shown in figures 25, 27, and 28.



Two samples were taken of the load, one on the crest of a dune, the other in the trough. The samples consisted of that material which did not pass through a 15-mm (0.05-foot) mesh screen. As one would expect from a type of sandwave that exhibits scour on the downstream side and deposition on the upstream, the rates of transport for trough and crest differed. The rates of transport of pebbles and cobbles were about 50 pounds per minute per foot width for the trough and about 20 pounds for the crest, based on 12-second sample periods. Such short sample periods can give only an estimate of the amount of material in motion. The difference of about 30 pounds would represent material deposited on the front of the dune and the 20 pounds in transport on the crest would represent material being carried through the reach. Because material is carried through the section without stopping, the rate of movement of antidunes cannot be used to estimate the amount of material in transport as bed load, as is sometimes done in the case of dunes and ripples. (Simons and others, 1961.)

The median diameter of material greater than 0.05 foot in diameter for the trough sample was 27 mm, that for the crest sample was 24 mm. This small difference determined from only one set of samples is not significant. Most of the material in transport was thought to be sand.

Often the antidunes appeared to remain stationary or to move only a foot or two upstream before dying out and being replaced by a new set. Measurements of the rate of movement were not made owing to the discontinuous nature of the movement, but were estimated at 1 to 2 feet per minute.

#### ANALYSIS OF COMPETENCE OF THE WHITE RIVER

Attempts to measure the largest materials in transport were more successful than attempts to measure bed load using the same screens. Some of the largest boulders were caught by hand, and if they could not be lifted from the water, they were rolled onto bars for measurement. Measurements were made of the largest boulders deposited on bars and the velocity in the channel immediately upstream from these bars. The wide range of materials available to White River provided an excellent opportunity to measure competence as all sizes at or near the range of competence of the stream were readily available. For a summary of the size ranges see table 2.

Figure 30 is a graph of the maximum size particle moving at a given velocity. Several factors that must be kept in mind when using such graphs are: the method of velocity measurement, the condition of the bed over which the particle was transported, and the conditions under which the data were gathered.

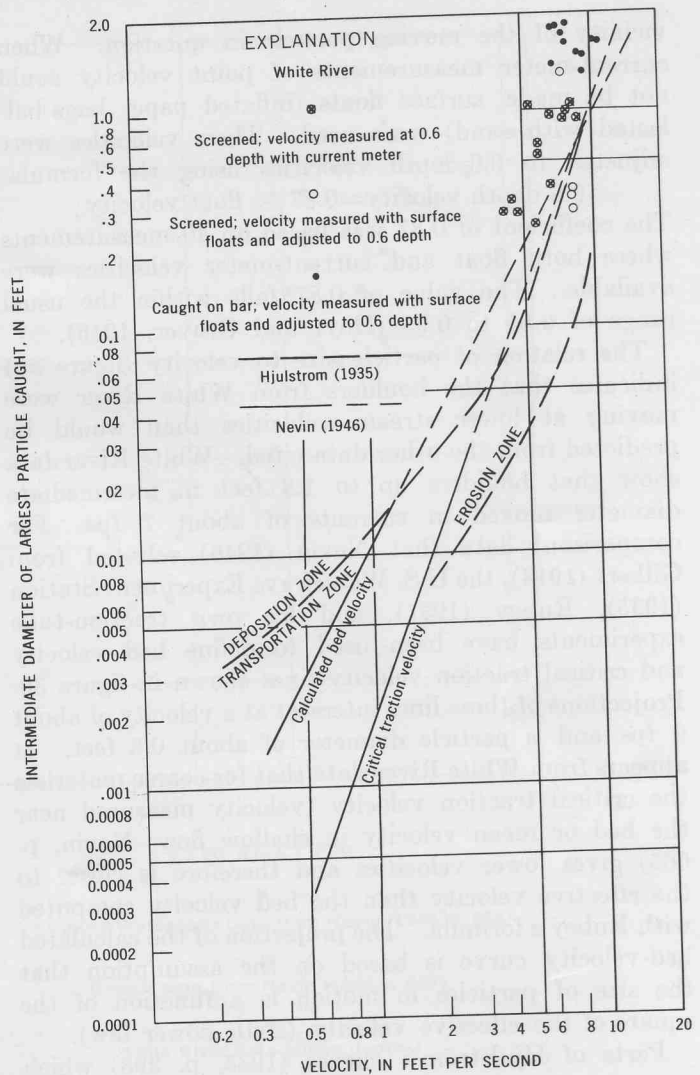


FIGURE 30.—Relation of particle size to velocity.

Velocities must not only be accurately measured, but must be comparable, preferably the velocities actually impinging on the particle.

Methods used to measure velocity ranged from the use of surface floats and current meters to mean velocities based on measurements of cross-sectional area of flow and discharge. Most of the values for White River point velocities were obtained by measurements with a Price type-A current meter set at 0.6 depth (normal setting for average velocity in a vertical section of a shallow turbulent stream). These current-meter velocities for White River were taken over a period of 20 seconds or a little more, about the practical maximum for the shifting channels being measured. As the greatest depth for any velocity observation was 2.1 feet, the maximum setting of the current meter was 0.8 foot from the bottom. As the particles in transit were up to 1.8 feet in intermediate diameter, the velocities measured were those in the immediate

vicinity of the moving particle in question. When current-meter measurements of point velocity could not be made, surface floats (inflated paper bags ballasted with sand) were used. Their velocities were adjusted to 0.6-depth velocities using the formula:

$$0.6 \text{ depth velocity} = 0.87 \times \text{float velocity}$$

The coefficient of 0.87 was based on 40 measurements where both float and current-meter velocities were available. The value of 0.87 falls within the usual range of 0.85 to 0.95 (Hoyt and Grover, 1916).

The relation of particle size to velocity (figure 30) indicates that the boulders from White River were moving at lower stream velocities than would be predicted from the other data cited. White River data show that boulders up to 1.8 feet in intermediate diameter moved in currents of about 7 fps. For comparison, data that Nevin (1946) selected from Gilbert (1914), the U.S. Waterways Experiment Station (1935), Rubey (1937), and his own traction-tube experiments have been used to define bed velocity and critical traction velocity lines shown in figure 30. Projections of these lines intersect at a velocity of about 9 fps and a particle diameter of about 0.8 feet. It appears from White River data that for coarse materials the critical traction velocity (velocity measured near the bed or mean velocity in shallow flow, Nevin, p. 665) gives lower velocities and therefore is closer to the effective velocity than the bed velocity computed with Rubey's formula. The projection of the calculated bed-velocity curve is based on the assumption that the size of particles in motion is a function of the square of the effective velocity (sixth power law).

Parts of Hjulström's curves (1935, p. 298) which separate the zones of erosion, transportation, and deposition are also shown in figure 30. Most data for White River boulders in motion plot in the zone of deposition that is defined by projections of these curves; however, these curves were based on average velocity and uniform bed materials.

Hjulström's curves, based on Schaffernak's (1922, p. 14) data for uniform materials from 8 to 70 mm, indicate that size is proportional to the 2.6 power of velocity, the power found by Nevin (1946), who used average velocity. Such a line (slope of 2.6) fits the White River data better than one with a slope of 2.0 (the "sixth power law" expressed in terms of linear dimensions); thus, it is suggested that for materials having intermediate diameters larger than 0.1 foot, streams may have greater competence than some applications of the "sixth power law" would predict.

The White River has larger particles in motion than would be predicted by either of the curves shown in figure 30 because the measured velocities were transporting and not necessarily eroding velocities. Most experi-

ments related to erosion and traction velocities have been made in laboratory flumes with uniform materials less than 0.1 foot in diameter, usually sand. The few experiments that have been made with mixed grain sizes show that the mobility of such materials is quite different from that of uniform materials. Gilbert (1914, p. 178) stated:

\*\*\* when such hollows are partly filled by the smaller grains its [the coarser particle's] position is higher and it can withstand less force of current. In other words, the larger particles are moved more readily on the smoother bed \*\*\*. The promotion of mobility applies not only to the starting of the grain but to its continuance in motion.

Ippen and Verma (1953) noted a similar effect of bed roughness. Thus because of the large range of particle sizes in the bed and banks of White River channels, one can expect the erosion of particles larger than those predicted by formulas based on data for uniform materials.

Another factor that must be considered when attempting to determine the significance of the largest boulders within a deposit are the methods by which such boulders may be set in motion by velocities far less than those that would disturb them from a bed composed solely of boulders of the same size. Some of these methods have not been considered in laboratory experiments. From the pictures of the White River source materials (figs. 14-22) it is obvious that coarse materials are not limited to the bed of the stream. Boulders of all sizes are poised in and on the cutbanks, ready for launching by any current that can erode the finer supporting materials. The relative instability of bank materials when compared to bed materials is due to the large gravity component which tends to make the material roll or slide down the bank. This gravity force component acting with force of the flow produces a resultant force much larger than the force acting on similar materials on the bed. Lane (1955a) suggests that the force on the bank materials be taken as the limiting force for stable canal design. Boulders on the bed may be set in motion by the blows from other boulders loosened from the banks. Finer materials may be scoured from under boulders setting them in motion. Boulders that are not moved from the bed on one day may tumble into the stream from the cutbank of the next day's channel as a result of the rapid cutting and filling that takes place in the White River. Illustrations of the rapidity of this change will be given in the section on pattern change (fig. 39).

The relation of size (fig. 31) to tractive force is similar to the relation of size to velocity as the sampled materials were coarser than would be predicted from experimental data on uniform materials. Lane's (1955a) data on the  $D_{75}$  (the diameter for which 75 percent of the material is smaller) of the material through which stable canals were constructed falls in a

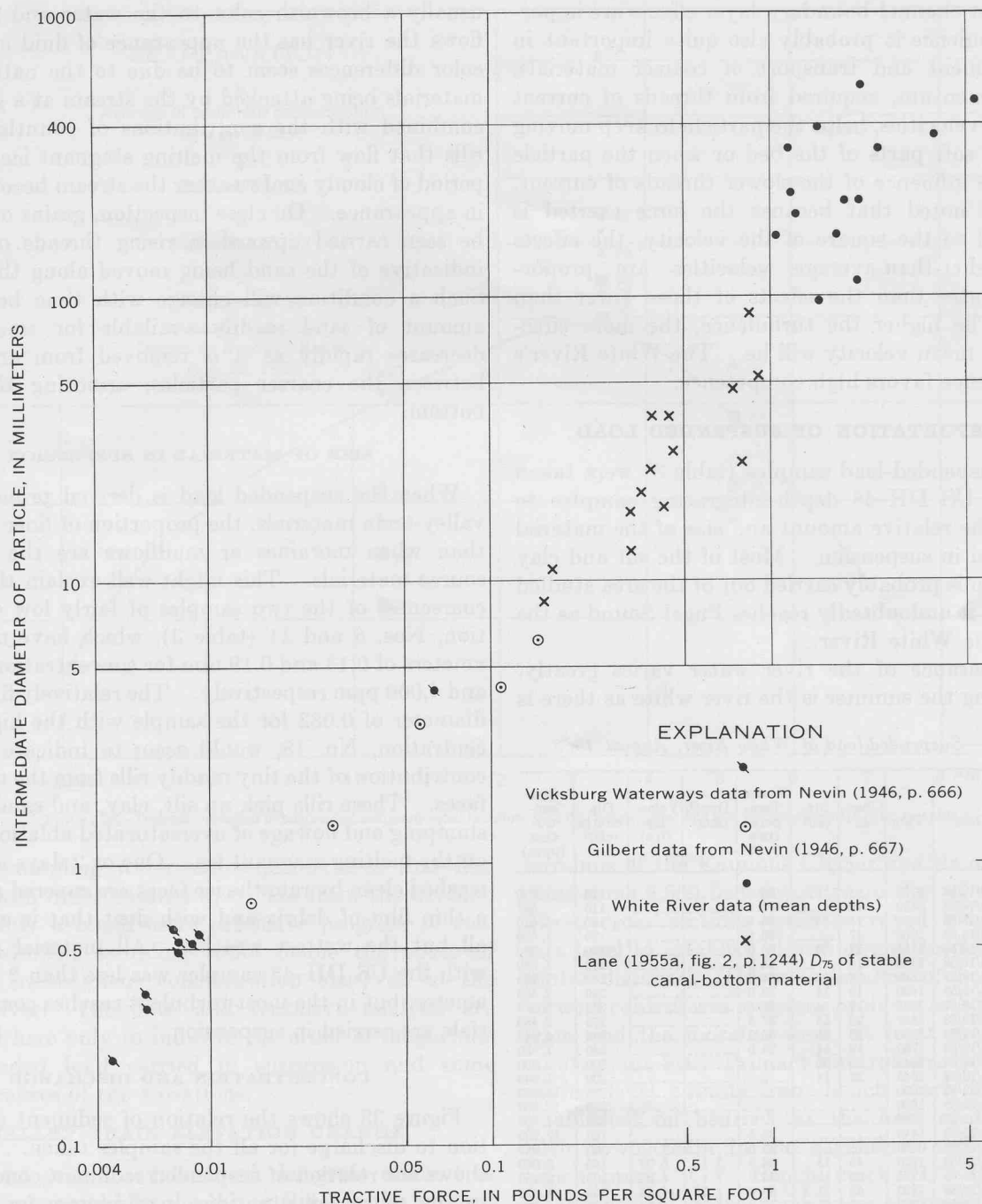


FIGURE 31.—Relation of particle size to tractive force.

line between the tractive forces calculated by Nevin (1946) and the White River data, suggesting that the total increase in mobility due to presence of mixed sizes must be limited.

Studies by White (1940) and by Kalinske (1942) emphasized the importance of turbulence in the initiation of motion of particles on the bed of natural streams. White (p. 332) stated:

If the turbulence extends up to and into the walls \* \* \* then a speed variation of 2 to 1 implies a stress variation of 4 to 1, and the maximum drag is four times the mean.

The so called effective velocity and the tractive force are only indicators of the degree of turbulence and of the probable maximum values of the velocity which may impinge upon a particle. Turbulence may well play the dominant role in the entrainment of materials



so small that channel boundary layer effects are important. Turbulence is probably also quite important in the entrainment and transport of coarser materials, for the momentum, acquired from threads of current with higher velocities, helps the particle to keep moving in rough or soft parts of the bed or when the particle is under the influence of the slower threads of current. It must be noted that because the force exerted is proportional to the square of the velocity, the effects of the higher-than-average velocities are proportionally greater than the effects of those lower than average. The higher the turbulence, the more effective a given mean velocity will be. The White River's high turbulence favors high competence.

#### TRANSPORTATION OF SUSPENDED LOAD

A few suspended-load samples (table 3) were taken by using a US DH-48 depth-integrating sampler to determine the relative amount and size of the material being carried in suspension. Most of the silt and clay in suspension is probably carried out of the area studied and some of it undoubtedly reaches Puget Sound as the "milk" of the White River.

The appearance of the river water varies greatly. Rarely during the summer is the river white as there is

TABLE 3.—*Suspended load of White River, August 1958*

Sample No.	Date	Time	Channel <sup>1</sup>	Station <sup>2</sup>	Water Temperature (° F)	Depth (feet)	Velocity (fps)	Discharge (cfs) <sup>3</sup>	Concentration (ppm)
1.	8/19/58	0720	-----	-----	36.5	-----	-----	120	520
2.	8/19/58	0810	18	-----	36.5	-----	-----	110	687
3.	8/19/58	0950	18	54	36.5	-----	-----	180	2,610
4.	8/19/58	1005	17	-----	36.5	-----	-----	180	1,650
5.	8/19/58	1135	17	14	38.0	-----	-----	220	1,860
6 <sup>4</sup>	8/19/58	1145	18	-----	38.0	-----	-----	240	3,630
7 <sup>5</sup>	8/19/58	1330	20	55	38.0	-----	-----	240	2,985
8.	8/19/58	1330	19	14	38.0	-----	-----	240	2,360
9.	8/19/58	1545	19	14	37.5	-----	-----	270	3,070
10.	8/19/58	1547	20	55	37.5	-----	-----	270	3,480
11 <sup>6</sup>	8/19/58	1730	20	55	36.0	-----	-----	240	4,049
12.	8/19/58	1730	19	14	36.0	-----	-----	240	1,830
13.	8/19/58	1945	23	55	-----	-----	-----	200	2,580
14.	8/19/58	1945	22	14	-----	-----	-----	200	1,030
15.	8/22/58	1650	-----	-----	-----	-----	-----	320	14,600
16.	8/22/58	1650	-----	-----	-----	-----	-----	320	15,500
17.	8/22/58	1700	-----	-----	-----	-----	-----	320	14,100
18 <sup>7</sup>	8/25/58	1445	-----	-----	36.5	-----	-----	700	17,220
19.	8/28/58	0830	-----	-----	35.0	-----	-----	200	989
20.	8/28/58	1220	46	15	36.0	1.1	6.02	165	2,060
21.	8/28/58	1225	46	15	36.0	1.1	6.02	165	2,180
22.	8/28/58	1220	46	21.5	36.0	1.35	4.46	165	1,410
23.	8/28/58	1225	46	21.5	36.0	1.35	4.46	165	1,540
24.	8/28/58	1215	46	27	36.0	1.7	4.51	165	1,060
25.	8/28/58	1225	46	27	36.0	1.7	4.51	165	1,260
26.	8/28/58	1520	47	17	37.0	1.4	5.96	190	3,190
27.	8/28/58	1520	47	17	37.0	1.4	5.86	190	4,550
28.	8/28/58	1520	47	22	37.0	1.4	5.86	190	2,680
29.	8/28/58	1520	47	22	37.0	1.4	5.86	190	2,930
30.	8/28/58	1520	47	28	37.0	1.6	6.29	190	2,490
31.	8/28/58	1520	47	28	37.0	1.6	6.29	190	2,720
32.	8/28/58	1715	48	15	36.0	1.2	6.97	165	2,800
33.	8/28/58	1715	48	20.5	36.0	1.3	5.52	165	2,190
34.	8/28/58	1715	48	26	36.0	1.6	4.50	165	1,910
35.	8/28/58	1715	48	30	36.0	1.5	5.63	165	1,560

<sup>1</sup> Sampling done before or after discharge measurement in channel described in table 7.

<sup>2</sup> Station number refers to a point on the channel cross section.

<sup>3</sup> Discharge estimated from gage height record and is given for sum of all channels.

<sup>4</sup> Median diameter of 0.13 mm.

<sup>5</sup> Median diameter of 0.066 mm.

<sup>6</sup> Median diameter of 0.19 mm.

<sup>7</sup> Median diameter of 0.082 mm.

usually a brownish color to the water and at highest flows the river has the appearance of fluid mud. The color differences seem to be due to the nature of the materials being attacked by the stream at a given time combined with the contributions of countless muddy rills that flow from the melting stagnant ice. After a period of cloudy cool weather the stream becomes milky in appearance. On close inspection, grains of sand can be seen carried upward in rising threads of current, indicative of the sand being moved along the bottom. Such a condition will change with time because the amount of sand readily available for traction load decreases rapidly as it is removed from around and between the coarser particles, armoring the stream bottom.

#### SIZE OF MATERIAL IN SUSPENSION

When the suspended load is derived primarily from valley-train materials, the proportion of finer size is less than when moraines or mudflows are the dominant source materials. This might well explain the relative coarseness of the two samples of fairly low concentration, Nos. 6 and 11 (table 3), which have median diameters of 0.13 and 0.19 mm for concentrations of 3,600 and 4,000 ppm respectively. The relatively fine median diameter of 0.082 for the sample with the highest concentration, No. 18, would seem to indicate the large contribution of the tiny muddy rills from the melting ice faces. These rills pick up silt, clay, and sand from the slumping and flowage of oversaturated ablation moraine on the melting stagnant ice. One or 2 days after being washed clean by rain the ice faces are covered again with a thin film of debris and with dust that is airborne in all but the wettest weather. All material picked up with the US DH-48 sampler was less than 2 mm in diameter, but in the most turbulent reaches coarser materials are carried in suspension.

#### CONCENTRATION AND DISCHARGE

Figure 33 shows the relation of sediment concentration to discharge for all the samples taken. Figure 32 shows the relation of suspended sediment concentration to the diurnal fluctuations in discharge for a day of moderate flow. The change in sediment concentration where the stream was divided into two channels of unequal discharges is shown in figure 34. Approximately one-fifth of the total flow is in the smaller channel. The samples were taken about 50 feet downstream from the point of division. It must be noted that the differences in sediment concentrations of samples taken at about the same time in large and small channels (fig. 34) are about the same magnitude as the variations across the channel (fig. 32); thus, the variation could be the result of the change in discharge due to the division, or the

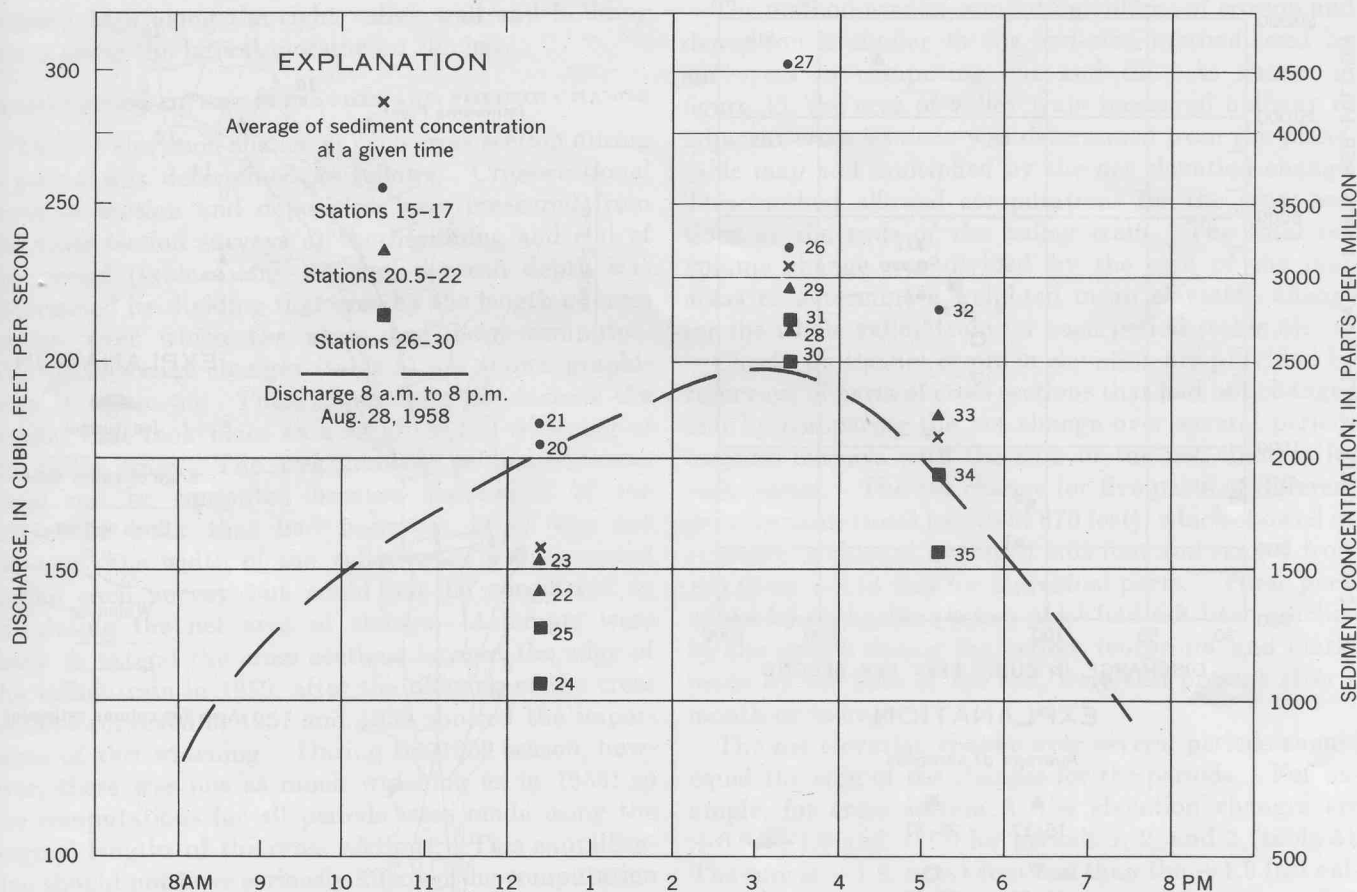


FIGURE 32.—Diurnal variation in suspended sediment concentration with discharge. Stations and sample numbers refer to table 3.

result of sampling water-sediment mixtures that had already been differentiated upstream from the division point. Only a much more extensive program of suspended-sediment sampling could define the cause of variation in sediment concentration observed in the White River. The data and tentative analysis are presented here only to indicate the order of magnitude of suspended load carried in suspension and some possible causes of the variations.

#### VALLEY-TRAIN ELEVATION CHANGE

The most graphic evidence of the great amount of material transported by the White River is the amount of erosion and deposition on the valley train itself. Measurements of the elevation changes due to erosion and deposition can give only minimum figures for transport because much of the material once set in motion is carried from the area. Measurements of elevation change are used later, in the study of the significance of channel pattern and in the study of the conditions of equilibrium.

#### METHODS OF MEASUREMENT

During late August, 1957, cross sections were surveyed across the White River valley train between the

terminus of the Emmons Glacier and its enclosing moraine some 6,600 feet downstream (fig. 35 and table 4). These cross sections were surveyed using a transit as a level to establish elevation and to determine horizontal distance. The maintenance of horizontal and vertical control was a serious problem as both the valley train and the moraine were, at least in part, resting on stagnant ice. Primary control was set up by surveying level circuits from bench mark 100 (fig. 35), established on bedrock at the base of Baker Point, both downstream to the gagehouse and upstream to bench mark "C." Bench mark "C" (fig. 35) was established on bedrock near the channel of West Emmons Creek by the National Park Service for use in their studies of Emmons Glacier. During the surveying of the line of levels, the elevations of a series of reference points near each cross section were established.

On subsequent surveys of cross sections, the height of instrument, referred to as HI, was established by computing an HI from each "known" reference point in the vicinity and taking the average of those that agreed within a tenth of a foot. Differential settling was assumed when the movement was more than 0.1 foot as the reference points were usually separated by

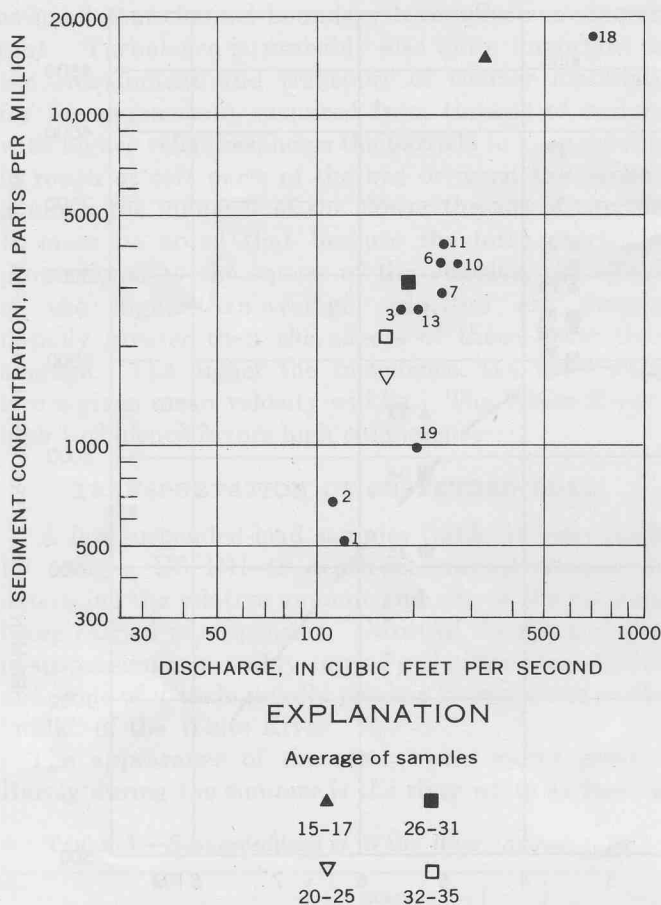


FIGURE 33.—Relation of suspended sediment concentration to discharge. Stations and sample numbers refer to table 3.

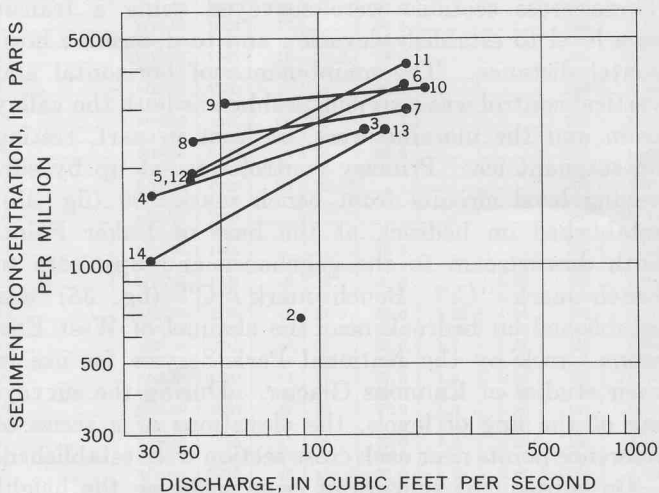


FIGURE 34.—Relation of suspended sediment concentration to discharge 50 feet down stream from the division into two channels. Lines connect samples taken at same time. Stations and sample numbers refer to table 3.

20 feet or more. Points showing movement were not used. Horizontal control was established by: (a) driving center stakes; (b) locating cross sections so

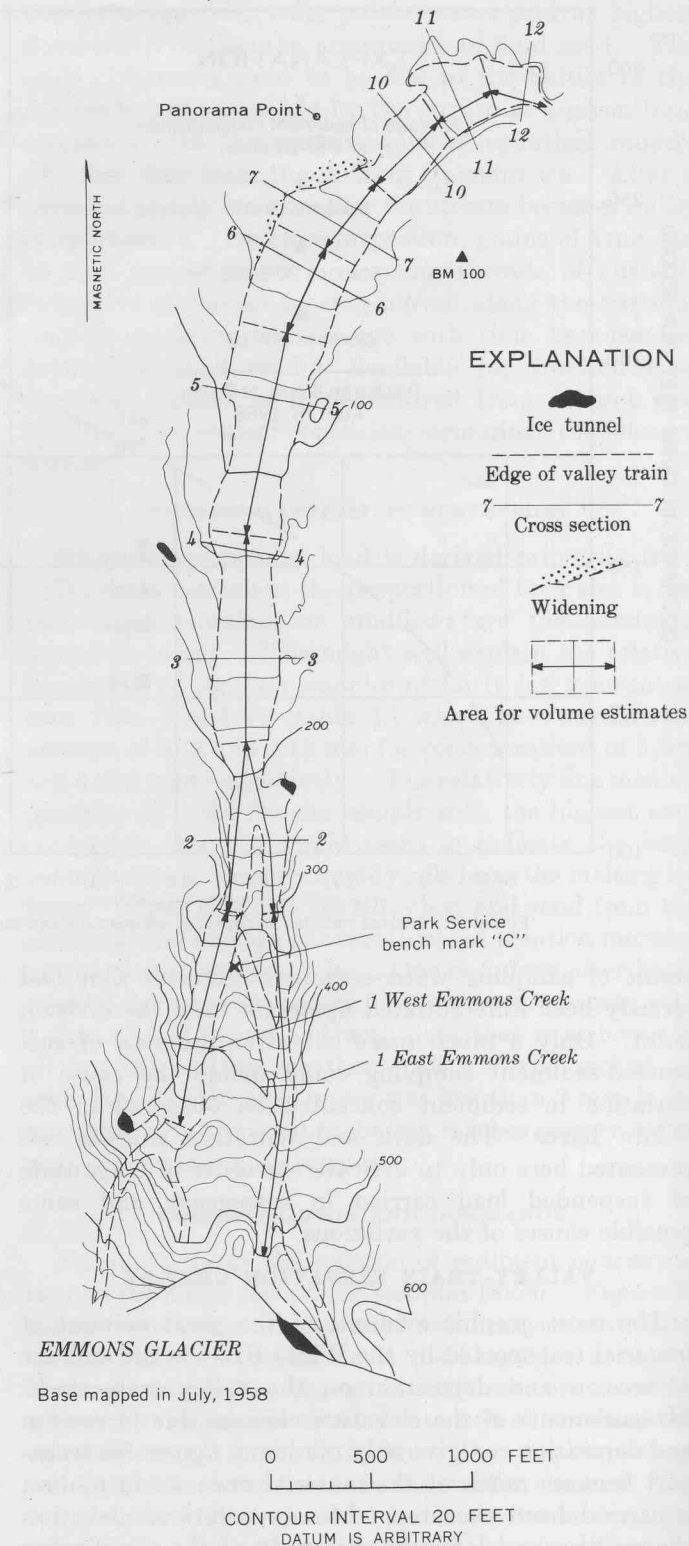


FIGURE 35.—White River valley train showing locations of cross sections, areas for volume estimates, and widening.

that reference points could be painted on large boulders along the line, and beyond the ends of the line, (c) measuring magnetic azimuths; (d) painting marks on



bedrock high along the right valley wall and building cairns along the lateral moraine on the left.

#### COMPUTATION OF NET ELEVATION AND VOLUME CHANGE

The net elevation change at each cross section during a period was determined as follows. Cross-sectional areas of erosion and deposition were measured from the cross section surveys at the beginning and end of the period (table 4; fig. 37) and a mean depth was determined by dividing that area by the length of cross section over which the areas had been computed. The net elevation changes (table 5) are shown graphically in figure 36. These changes do not include the erosion that took place as a result of the widening of the valley train. The area involved in this widening could not be computed because the height of the terrace or bank that had been cut away was not known. The width of the valley train was measured during each survey but could not be considered in calculating the net area of change. Attempts were made to extend the cross sections beyond the edge of the valley train in 1959, after the plotting of the cross sections surveyed in 1957 and 1958 showed the importance of this widening. During the 1959 season, however, there was not as much widening as in 1958; so the computations for all periods were made using the original lengths of the cross sections. This simplification should not have seriously affected the computation of net elevation change, as similar elevation changes took place on the newly added part of a cross section. The changes in length of cross sections are given in table 4.

The method used in computing volume of erosion and deposition is similar to the end-area method used by surveyors in computing cut and fill. As shown in figure 35, the area of valley train measured halfway to adjacent cross sections was determined from the plane-table map and multiplied by the net elevation change. This method allowed computations for the cross sections at the ends of the valley train. The total net volume change was divided by the sum of the map areas to determine a weighted mean elevation change for the whole valley train for each period (table 5).

Checks on the net errors in elevation are provided by resurveys of parts of cross sections that had not changed and by comparing the net change over several periods between surveys with the sum of the net changes for each period. The net change for five parts of different cross sections (total length of 670 feet), which showed no evidence of change, averaged 0.03 foot and ranged from +0.12 to -0.13 foot for individual parts. These parts where no change had taken place had not been reached by the stream during the period; footprints, and marks made by the base of the rod, were still present after a month or more.

The net elevation change over several periods should equal the sum of the changes for the periods. For example, for cross section 3, the elevation changes are +0.9, +1.9 and -1.0 for periods 1, 2, and 3 (table 5). The sum is +1.8, or 0.1 foot less than the +1.9 feet calculated for period 4, the sum of periods 1, 2, and 3. Because periods 4, 8, 9, and 10 are composed of periods of shorter duration, similar checks are provided for all periods at each cross section and for the sum of the

TABLE 4.—Cross section survey of the White River valley train, 1957-60

Cross section No. (fig. 35)	Duration of periods and dates of surveys								Valley train					
									Dimensions			Distance from glacier in 1958 (feet)	Widening (feet)	
	Width (feet)	Slope (feet per foot)	Surface area (square feet)	Period 4	Period 9									
Period 10														
Period 4				Period 9										
Period 8														
Period 1	Period 2	Period 3	Period 5	Period 6	Period 7	Period 11								
0					7/ 1/59		8/19/59	115	0. 16		1 3, 000		0	
1W					7/ 3/59		8/19/59	162	. 14		11, 000		10	
1E					7/ 1/59		8/19/59	182	. 16		1, 250		0	
2	8/ 1/57	7/15/58	8/ 5/58	9/ 2/58	6/21/59	7/26/59	8/25/59	240	. 11	180, 000	2, 350	50	2—52	
3 s	8/28/57	7/15/58	8/ 5/58	9/ 2/58	6/21/59	7/26/59	8/25/59	290	. 075	320, 000	3, 500	65	14	
4					6/21/59	7/26/59		300	. 06		3, 700		0	
5					6/20/59	7/25/59	8/26/59	425	. 05	340, 000	4, 500		0	
6	8/22/57	7/15/58	8/ 6/58	8/29/58	6/20/59	7/25/59	8/26/59	435	. 04	200, 000	5, 150	34	2—14	
7	8/21/57	7/ 2/58	8/ 6/58	8/28/58	6/20/59	7/25/59	8/26/59	440	. 04	150, 000	5, 400	74	470	
8	8/19/57	7/ 7/58	8/ 6/58	8/28/58	6/20/59	7/25/59	8/26/59	290	. 03	110, 000	6, 000		0	
9	8/20/57	7/ 8/58	8/ 7/58	8/25/58	6/19/59	7/24/59	8/25/59	360	. 03	110, 000	6, 250	0	0	
10	8/20/57	7/ 1/58	8/ 8/58	8/28/58	6/19/59	7/24/59	8/26/59	170	. 03	50, 000	6, 600	0	0	
11	8/20/57	7/ 9/58	8/ 8/58	8/28/58	6/19/59	7/24/59	8/26/59	151	. 055		11, 600		0	
12	8/20/57						8/19/59	6/ 1/60			78, 500		0	
13							8/20/59	6/ 3/60					0	
14				9/ 4/58										

TABLE 5.—Changes in elevation and volume of the valley train of the White River, 1957–60

[See table 3 for dates of periods and figure 36 for plot of data for cross sections 2–12. Italic figures show discrepancy as compared to the sum of the individual periods.]

Period No.-----	1	2	3	4 (1+2+3) 12	5	6	7	8 (6+7) 2	9 (5+8) 12	10 (4+9) 24	11
Duration (months)-----	10	1	1		10	1	1				10
Elevation change (feet)											
Cross section											
0-----								-1.2			
1W-----								-5			0
1E-----								-2			+4
2-----	-0.6	-0.1	-0.2	-0.9	+0.5	-0.2	-0.3	-4	+0.2	-0.7	+3.3
3-----	+9	+1.9	-1.0	+1.9	-2	-1	-2	-3	-5	+1.4	+2.4
4-----						+4	0	+3			+1.4
5-----	+1.1	+9	0	+2.0	+4	-2	-3	-4	0	+2.1	+6
6-----	+9	+1	+9	+1.9	+1	-4	-1	-5	-4	+1.5	+4
7-----	+1.0	+8	-3	+1.4	+9	-3	+1	-1	+6	+2.0	+9
10-----	+3	-3	0	0	-3	+7	+1	+7	+3	+4	+6
11-----	0	+3	0	+3	-3	+2	+2	+3	+1	+3	-5
12-----	+5	+4	+2	+1.1	-2	-5	+7	+1	0	+1.1	-4
13-----									0		-1.0
14-----									-2		
Total volume change (10 <sup>3</sup> cubic feet) <sup>2</sup>											
All cross sections-----	+900	+1, 100	-200	+1, 800	+300	-200	-200	-400	-200	+1, 700	+1, 800
Average elevation change (feet) <sup>2</sup>											
All cross sections-----	+0.6	+0.7	-0.2	+1.2	+0.2	-0.1	-0.1	-0.3	-0.1	+1.2	+1.2

<sup>1</sup> Survey in midperiod showed deposition of 3.6; subsequent erosion of 1.7 gave net for period of 1.0.<sup>2</sup> Total volume change and average elevation change are based on cross sections 2, 3, 5–7, 10–12.

net changes of all cross sections. The italicized values in table 5 for periods 4, 8, 9, and 10 are those which differed by 0.1 foot from the sum of the included periods. An error of 0.1 foot can be produced by rounding the calculations to tenths. The frequency with which an error of 0.1 occurs, as well as the limit in accuracy imposed by the ruggedness of the terrain surveyed, suggests that 0.1 foot is about the limit of accuracy for procedures outlined. The fact that larger errors are infrequent (bold type in table 5) attests to the general reliability of the procedure used to determine elevation change.

In estimating volume change there is also the possibility of error in the determination of area. No check is available for this. In addition, the assumption has to be made that the elevation change for the cross section applies to all points within the area of valley train (fig. 35) in which the cross section is centered. The main purpose of volume determination is the calculation of an average elevation change for the entire area weighted to allow for differences in spacing of the cross sections. The discrepancies that appear when the volume changes for cumulative periods are compared with the sum of those for the component periods are the same discrepancies noted in the section on elevation change.

#### ELEVATION CHANGE ON CROSS SECTIONS OF THE VALLEY TRAIN

Eleven cross sections (table 4) were surveyed in 1957 (figs. 35, 36, 37). When resurveyed in early July 1958, it was discovered that two-thirds of cross section 0 had been overridden by a slight advance of the glacier. The controls for Nos. 4 and 8 had been eroded or buried and these cross sections could not be resurveyed. Although other controls were lost, enough points remained or were reestablished to allow all other cross sections to be resurveyed with reasonable certainty that the elevation changes were not due to faulty location or faulty HI.

These 8 cross sections (Nos. 2, 3, 5–7, 10–12) were resurveyed 3 times in 1958—between July 1 and 15, between August 5 and 8, and between August 28 and September 2—and 3 times in 1959—between June 19 and 21, July 24 and 26, and August 25 and 26. No. 3 was also surveyed on July 25, 1958, by plane-table methods because striking deposition had taken place (fig. 37). In addition, 4 new cross sections were established, 3 of them upstream from No. 2 and the 4th in the vicinity of No. 4, which had been lost in 1958 (fig. 35). Nos. 13 and 14, established on September 4, 1958, were resurveyed on August 19 and 20, 1959. Dates on which each cross section was surveyed are

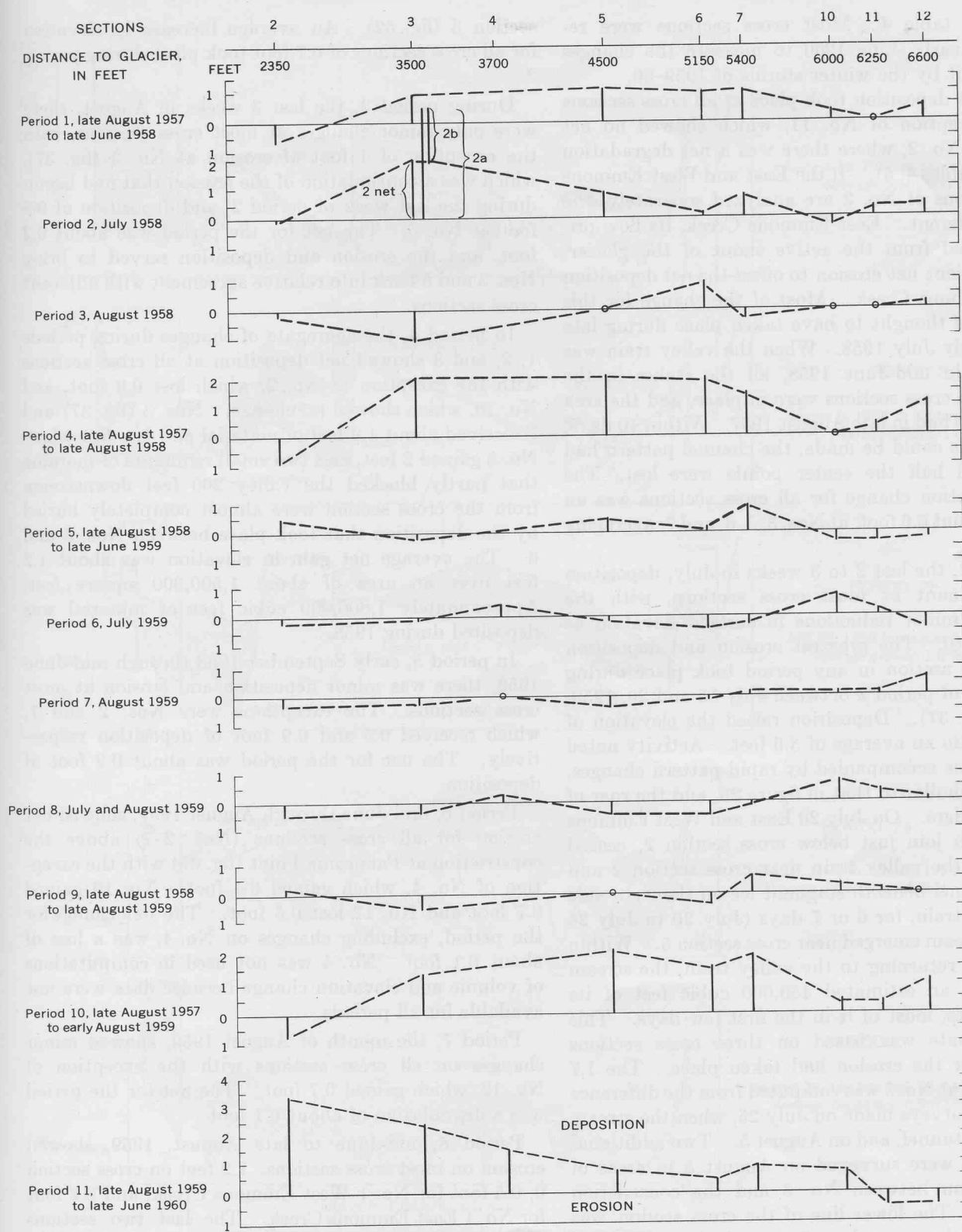


FIGURE 36.—Erosion and deposition on cross sections. For exact dates of surveys for each section, see table 4.



indicated in table 4. Most cross sections were re-surveyed in early June 1960 to measure the changes brought about by the winter storms of 1959-60.

In period 1 deposition took place at all cross sections with the exception of No. 11, which showed no net change, and No. 2, where there was a net degradation of 0.6 foot (tables 4, 5). If the East and West Emmons Creeks portions of No. 2 are analyzed separately, the picture is different. East Emmons Creek, its flow primarily derived from the active snout of the glacier, showed sufficient net erosion to offset the net deposition of West Emmons Creek. Most of the change for this period is thus thought to have taken place during late June and early July 1958. When the valley train was first visited in mid-June 1958, all the stakes in the centers of the cross sections were in place, and the area appeared as it had in late August 1957. Within 10 days, before surveys could be made, the channel pattern had changed, and half the center points were lost. The average elevation change for all cross sections was an increase of about 0.6 foot, at Nos. 3, 5, 6, and 7, averaging about 1 foot.

In period 2, the last 2 to 3 weeks in July, deposition was predominant at most cross sections, with the exception of minor reductions in surface elevation at Nos. 2 and 10. The greatest erosion and deposition for any cross section in any period took place during the first part of period 2 between July 15 and 20, 1958, at No. 3 (fig. 37). Deposition raised the elevation of the valley train an average of 3.6 feet. Activity noted on July 17 was accompanied by rapid pattern changes, rough water similar to that in figure 26, and the roar of clashing boulders. On July 20 East and West Emmons Creeks, which join just below cross section 2, ceased flowing over the valley train near cross section 2 and entered a tunnel beneath stagnant ice on the right side of the valley train, for 6 or 7 days (July 20 to July 26 or 27); the stream emerged near cross section 5. Within 10 days after returning to the valley train, the stream had removed an estimated 450,000 cubic feet of its earlier deposits, most of it in the first few days. This volume estimate was based on three cross sections surveyed after the erosion had taken place. The 1.7 feet of erosion at No. 3 was computed from the difference between the surveys made on July 25, when the stream was in the ice tunnel, and on August 5. Two additional cross sections were surveyed on August 5 in areas of obvious erosion between No. 3 and the constriction below No. 2. The lower line of the cross section was surveyed on the ground surface and the upper line was inferred from erosion remnants. On July 28, during the removal of this material, one-third of the valley train in this reach was covered with water flowing in at least 4 channels in the vicinity of cross

section 3 (fig. 52). An average increase in elevation for all cross sections of 0.7 foot took place during period 2.

During period 3, the last 3 weeks in August, there were only minor changes at most cross sections, with the exception of 1 foot of erosion at No. 3 (fig. 37), which was a continuation of the erosion that had begun during the last week of period 2, and deposition of 0.9 foot at No. 6. The net for the period was about 0.2 foot, and the erosion and deposition served to bring Nos. 3 and 6 back into relative agreement with adjacent cross sections.

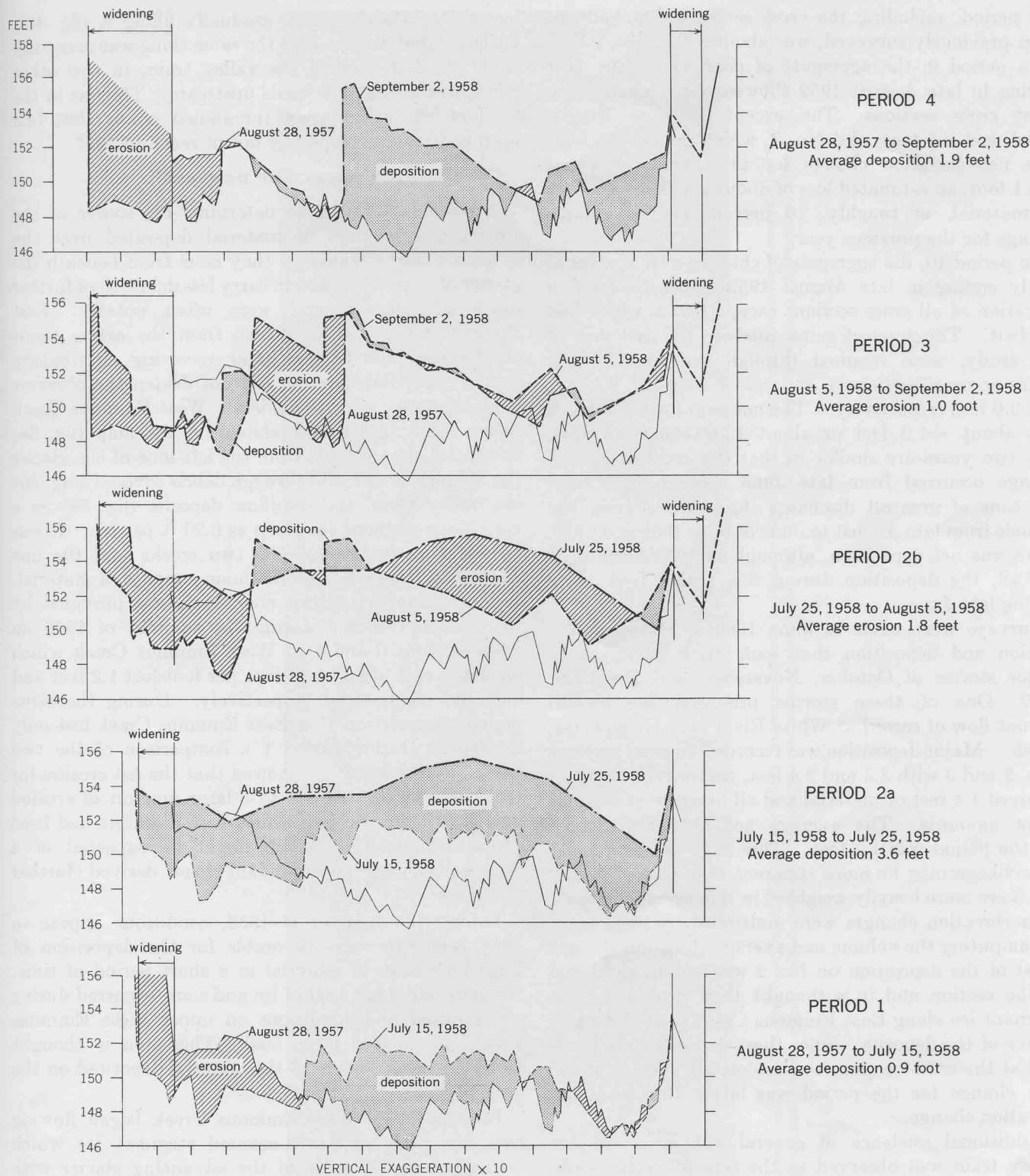
In period 4, the aggregate of changes during periods 1, 2, and 3 showed net deposition at all cross sections with the exception of No. 2, which lost 0.9 foot, and No. 10, which showed no change. Nos. 3 (fig. 37) and 7 received about 1.9 feet of material and No. 6, 1.4 feet. No. 5 gained 2 feet, and two small remnants of moraine that partly blocked the valley 200 feet downstream from the cross section were almost completely buried by the deposition that took place between Nos. 5 and 6. The average net gain in elevation was about 1.2 feet over an area of about 1,500,000 square feet. Approximately 1,800,000 cubic feet of material was deposited during 1958.

In period 5, early September 1958 through mid-June 1959, there was minor deposition and erosion at most cross sections. The exceptions were Nos. 2 and 7, which received 0.5 and 0.9 foot of deposition respectively. The net for the period was about 0.2 foot of deposition.

Period 6, mid-June through August 1959, showed net erosion for all cross sections (Nos. 2-7) above the constriction at Panorama Point (fig. 35) with the exception of No. 4, which gained 0.4 foot. No. 10 gained 0.7 foot and No. 12 lost 0.5 foot. The net change for the period, excluding changes on No. 4, was a loss of about 0.1 foot. No. 4 was not used in computations of volume and elevation change because data were not available for all periods.

Period 7, the month of August 1959, showed minor changes on all cross sections with the exception of No. 12, which gained 0.7 foot. The net for the period was a degradation of about 0.1 foot.

Period 8, mid-June to late August, 1959, showed erosion on most cross sections: 1.2 feet on cross section 0, 0.5 foot for No. 1 West Emmons Creek, and 0.2 foot for No. 1 East Emmons Creek. The last two sections were established to evaluate these reaches as possible sources for materials deposited in the valley train. Exceptions to the erosion were Nos. 4, 11, and 12, where minor deposition had taken place, and No. 10 which had received 0.7 foot of material. The net for



Note: Widening not included in computation of average elevation change

FIGURE 37.—Erosion and deposition on cross section 3 from August 28, 1957, thru September 2, 1958.

the period, excluding the cross sections that had not been previously surveyed, was about  $-0.3$  foot.

In period 9, the aggregate of changes for the year ending in late August 1959 showed minor changes on most cross sections. The exceptions were No. 3, which lost 0.5 foot, and No. 7, which gained 0.6 foot. The net elevation change for this year was about  $-0.1$  foot, an estimated loss of about 200,000 cubic feet of material, or roughly, 10 percent of the volume change for the previous year.

In period 10, the aggregate of changes over 2 years of study ending in late August 1959 showed a gain in elevation at all cross sections except No. 2, which lost 0.7 foot. The greatest gains, made in the first year of the study, were retained through the second year. Gains on cross sections 3, 5, 6, and 7 were 1.4, 2.1, 1.5, and 2.0 feet respectively. The net gain for the 2 years was about  $+1.2$  feet or about 1,700,000 cubic feet. The two years are similar in that the greatest rate of change occurred from late June through late July, the time of greatest discharge (fig. 50). During the periods from late August to June of both 1958 and 1959, there was net deposition, although in 1958 most of, if not all, the deposition during this period took place during late June.

Surveys were made in June 1960 to check on the erosion and deposition that took place during three major storms of October, November, and December 1959. One of these storms produced the second highest flow of record at White River near Greenwater, Wash. Major deposition was recorded on cross sections Nos. 2 and 3 with 3.3 and 2.4 feet, respectively. No. 4 received 1.4 feet of material and all other cross sections lesser amounts. The average net elevation change for the period was 1.2 feet. This large average elevation change may be more apparent than real as Nos. 2 and 3 are more heavily weighted in this average because their elevation changes were multiplied by large areas in computing the volume and average elevation change. Most of the deposition on No. 2 was on the right end of the section and it is thought that mudflows from stagnant ice along East Emmons Creek were the major source of the deposits. Since these deposits were localized at the cross section, the calculated average elevation change for the period was larger than the true elevation change.

Additional evidence of general alluviation of the valley train was observed in the fate of kettle pools. Morning Glory Pool, in the adjacent moraine, so-named for its beautiful color in 1957, still deserved this name in early June 1958. At this time it was brimming with potable clear water, the surface of which was several feet above the adjacent valley train. Periodic invasion by debris-laden streams built a birds-

foot delta into the pond, gradually filling it (fig. 40). By the end of August 1958 the same thing was occurring along the left edge of the valley train, in two other ponds in the next 300 yards upstream. The rise in the level of the valley train threatened ponds that had been considered completely out of reach in 1957.

#### SOURCE OF DEPOSITS

A search was made to determine the source of the 1,650,000 cubic feet of material deposited over the 2 years. The streams, as they issue from beneath the glacier, usually appeared to carry less debris than further down downstream and were often potable. East Emmons Creek, which flowed from the active snout of the Emmons Glacier after receiving a tributary from Frying Pan Glacier, did not deafen the observer with a din of clashing boulders. West Emmons Creek before reaching the area shown on the map (fig. 35) flowed for about 1 mile from the left lobe of the glacier (fig. 1) over, under, and through debris covered stagnant ice, valley train, and mudflow deposits (fig. 38) on a very steep gradient (as steep as 0.20 ft per ft). It was usually the muddier of the two creeks and the one which was thought to be the main supplier of material. Evidence supporting this conclusion was provided by the erosion recorded during the summer of 1959 on cross sections 0 and 1 of West Emmons Creek which on slopes of 0.16 and 0.14 foot per foot lost 1.2 feet and 0.5 foot of material respectively. During the same period cross section 1 on East Emmons Creek lost only 0.2 foot. During period 1 a comparison of the two creeks at cross section 2 showed that the net erosion for the cross section was due to a large amount of erosion by East Emmons Creek while West Emmons had been depositing material, which might be expected of a stream carrying an abundant load derived farther upstream.

During the summer of 1958, conditions appear to have been especially favorable for the deposition of large quantities of material in a short period of time. An unusually high melt of ice and snow occurred during the summer and conditions on upper West Emmons Creek provided a large load. This area is thought to be the major source of the material deposited on the valley train.

In June 1958, West Emmons Creek began flowing over the ridge of debris-covered stagnant ice which connected the left side of the advancing glacier with stagnant ice along the left side of the valley train. Prior to this diversion over the ice ridge, the stream had flowed relatively quietly under the ridge in one or more ice tunnels. These tunnels must have been blocked in early June by debris or collapse. They may have been blocked in part by materials from a mud-





FIGURE 38.—Valley train near head of West Emmons Creek, July 3, 1958. See also figure 1. Stream plunges into ice tunnel in center of picture. Two weeks earlier it flowed across the field of view. Note mudflow levees (parallel strings of boulders in right center). Valley train is almost completely enclosed and underlain by stagnant ice. As an indication of scale, the boulder in right foreground is 10 to 15 feet in diameter.

flow that had descended on top of snow, probably during the spring, and flowed as far as cross section 5, where remnants of the snow it had buried and protected were still melting in late June. When the stream began flowing over the ridge, it reached its former bed on the downstream side of the ice ridge after plunging in a spectacular cascade, called during field studies "chocolate falls" after the color of the water. This cascade was about 70 feet high at first but within a few days the stream with its load of debris had cut deeply into the ice. The rate of cutting was estimated at 5–10 feet per day. Within a few weeks the stream had cut down leaving a vertical-walled ice gorge shown in figure 1 and, in part on the map, figure 35. High on the sides of this gorge, parts of abandoned meanders could be seen at various levels. They appeared as debris-covered terraces or, when undercut, as shelves. The roar of the stream within the gorge made it impossible to be heard at its margin.

The high melt of late June, acting with the changes in gradient brought about by this stream diversion over the ice and the rapid erosion, delivered large quantities of debris to the upper reaches of the valley train. (Note the changes in cross sections 2 and 3 during the second period, table 5.) This material was subsequently removed and distributed over most of the valley train. The change in gradient (knickpoint) at the site of the falls was almost entirely removed by early August, and the lower discharges of August 1958 and of the summer of 1959, on the lowered gradient, brought far less material to the valley train.

Above the ice ridge, West Emmons Creek is very unpredictable, frequently disappearing under the valley train into previously unknown tunnels; this disappearance indicates that the valley train in this area is underlain by stagnant ice. One day it is a formidable stream to cross, and the next day it may not even be present on the surface (figure 38).

An additional source of material was provided by widening of the valley train by erosion of the deposits along its margins. The changes in width which were recorded for the 2 years are shown in table 4, and the area of maximum widening has been sketched on the map, figure 35. Here the channel widening reached a maximum of about 100 feet when the stream removed a bank 10 feet high and found access to an area of kettles behind.

Negative changes in length of cross section (table 4)—that is, narrowing—took place by slumping of banks and by the addition of material from the areas of stagnant ice. Widening of the valley train took place by lateral erosion and filling of depressions that had been separated from the valley train by moraine or mudflow deposits. The pond, well back from the river, shown

in the foreground of figure 39 A (photographed July 18, 1958), is the pond (elevation 56.0) at the edge of the valley train in figure 46, mapped September 3, 1958. The pond had become part of the valley train by the time the widening was mapped the next summer. The materials added to the valley train by this lateral erosion are estimated to make up about 10 percent of the total deposition. Since widening showed much less continuity than erosion or deposition, extrapolations to the areas between cross sections in order to calculate volume were not thought justified.

It is interesting to note that the maximum widening (fig. 35) took place not at the narrowest point of the valley train but at the outside of the valley curve toward the east, at the junction of the White River valley with the valley of Inter Fork (fig. 2). The coarser fraction of the large quantity of material furnished by stream cutting of the high banks in this area was deposited first as a bar within 100 yards downstream from the cut banks. This bar may well have served to protect the constriction downstream from erosion during the highest flows of the summer.

In summary, the abundant discharge and high gradients of both East and West Emmons Creeks, with assistance from unique conditions on West Emmons Creek and widening of the valley train, provided an abundant load during 1958. The high gradients on the Emmons Creeks with the flows of 1959 continued to supply materials to the valley train but in greatly reduced quantities, and erosion exceeded deposition for most of the valley train. There was a slight flattening of the valley profile, owing to the predominance of erosion above the junction of the two creeks and depositions over most of the downstream part of the valley train.

#### CHANNEL PATTERN

The description and analysis of the pattern change of White River channels presents a problem owing to the rapidity with which the changes take place. Slight changes in water-surface elevation which caused the stream to occupy or abandon old streambeds provided as much change in appearance of the channel pattern and valley train as major erosion and deposition (fig. 39). In order to analyze the mechanism of pattern change panoramic photographs (fig. 40) and time-lapse motion pictures were taken (Fahnestock, 1959), planetable maps and cross sections surveyed, and measurements of channel characteristics and elevation change were made.

#### DESCRIPTION OF PATTERN

##### CHANGES DURING THE PERIOD OF STUDY

To record the change in pattern of White River channels during the summers, panoramas like figure 40 were

TABLE 6.—Number of channels at each cross section of the White River, 1958-59

[Data from panoramas]

Date	Time	Discharge (cu ft/sec)	Cross section—							
			3	5	6	7	10	11	12	
1958										
June:										
18.....	0930	-----	1	3	2	5	3	3	2	
18.....	1830	-----	2	6	6	6	6	6	3	
19.....	1015	-----	2	5	6	6	5	5	3	
26.....	1040	190	1	2	5	9	3	2	3	
July:										
1.....	0905	100	1	1	3	6	2	1	3	
7.....	1020	210	2	2	6	7	2	1	2	
7.....	1945	310	2	3	5	6	4	2	3	
8.....	2030	320	2	4	8	8	6	3	2	
11.....	1220	410	2	3	7	5	2	1	4	
14.....	1400	240	2	3	6	5	5	5	2	
15.....	0945	200	1	4	3	3	2	2	2	
16.....	0915	260	1	2	7	5	3	1	1	
17.....	1715	370	3	6	8	7	6	4	3	
18.....	1050	340	3	2	11	13	7	5	2	
21.....	1525	330	10	3	5	7	3	1	2	
24.....	0745	370	10	3	2	6	5	4	3	
25.....	0740	370	10	2	5	6	4	4	2	
28.....	1220	430	4	6	7	5	3	2	2	
28.....	1900	600	4	7	4	5	6	5	2	
29.....	0745	430	3	3	6	4	6	4	1	
31.....	1030	320	1	2	6	6	2	2	1	
Aug:										
2.....	1005	210	1	5	4	5	2	1	1	
3.....	1920	210	1	4	3	3	1	1	1	
4.....	1230	195	2	3	2	3	2	1	1	
5.....	1900	280	1	5	4	4	2	1	1	
6.....	1810	370	1	4	5	5	3	2	3	
7.....	0830	220	1	4	6	5	5	3	2	
8.....	1100	210	2	4	9	7	4	3	2	
8.....	1900	340	2	5	8	9	3	2	2	
9.....	1500	490	1	3	7	10	4	2	2	
11.....	0910	300	1	5	6	6	1	1	2	
13.....	1245	310	1	1	4	6	1	1	1	
14.....	-----	-----	1	4	8	6	4	3	2	
15.....	1815	280	1	3	7	5	1	1	1	
18.....	1720	250	1	1	6	4	3	2	1	
19.....	1820	210	1	2	5	3	2	3	1	
20.....	0835	210	1	1	6	5	4	3	2	
22.....	1755	500	1	2	6	8	5	6	3	
25.....	0840	300	1	1	8	7	3	2	1	
28.....	0820	120	-----	2	7	8	2	1	2	
29.....	1120	170	1	2	8	7	2	2	2	
Sept:										
2.....	0830	170	1	2	7	6	1	1	2	
4.....	1545	180	1	2	9	7	1	1	2	
30.....	1130	100	1	1	2	2	1	1	2	

Date	Time	Discharge (cu ft/sec)	Cross section—							
			3	5	6	7	10	11	12	
1959										
June:										
18.....	1535	130	1	1	3	3	3	2	2	
20.....	0930	140	2	2	3	3	2	2	2	
26.....	1400	120	2	1	2	4	2	2	3	
30.....	1000	90	1	1	1	3	2	2	3	
July:										
7.....	1815	100	1	1	1	2	2	2	2	
9.....	1830	140	1	1	3	4	-----	-----	-----	
10.....	1415	140	1	2	3	4	-----	-----	-----	
11.....	-----	-----	2	1	3	4	3	3	3	
12.....	1630	190	2	2	4	5	3	3	3	
13.....	1600	150	2	2	4	4	4	3	3	
15.....	1230	160	1	1	3	5	3	3	4	
16.....	1910	230	1	1	3	6	-----	-----	-----	
17.....	2000	230	1	2	5	6	-----	-----	-----	
18.....	1325	250	2	2	3	3	3	3	4	
19.....	0940	220	1	2	2	4	4	3	4	
19.....	2000	420	1	3	4	5	3	4	3	
20.....	1250	370	2	3	4	4	2	2	2	
20.....	1930	290	2	4	4	5	4	3	3	
20.....	1930	280	2	3	4	6	3	2	3	
21.....	0915	430	1	2	5	5	4	5	2	
24.....	1940	420	-----	6	4	4	4	3	2	
24.....	1045	490	2	2	3	4	4	3	3	
26.....	1800	380	2	2	5	5	-----	-----	-----	
30.....	1315	230	2	3	5	5	5	4	3	
Aug:										
1.....	1815	280	2	3	3	4	-----	-----	-----	
3.....	1642	170	1	3	3	2	4	3	3	
4.....	1600	180	1	3	2	3	2	2	3	
5.....	1450	230	1	4	2	2	3	3	2	
6.....	1345	220	1	3	3	3	2	3	2	
7.....	1420	240	1	3	2	2	3	3	3	
8.....	1505	250	1	3	1	3	3	5	3	
9.....	1520	210	1	3	1	3	4	4	3	
10.....	1430	220	1	3	3	3	3	3	3	
11.....	1725	170	1	3	1	3	2	2	3	
13.....	1710	180	1	2	1	3	3	3	3	
14.....	1530	220	1	3	2	4	2	3	3	
17.....	1445	170	1	3	2	3	2	3	2	
18.....	1505	140	1	3	2	3	2	2	2	
20.....	0945	100	1	2	2	3	2	2	2	
24.....	1000	150	1	2	2	2	3	2	2	
25.....	1035	120	1	1	3	2	-----	-----	-----	
28.....	1705	160	1	2	2	3	2	2	2	
29.....	1510	180	1	2	2	3	3	2	3	
Nov:										
27.....	1200	40	1	2	3	2	3	3	2	

<sup>1</sup> The channel at cross section 3 was flowing through an ice tunnel.

taken as frequently as several times per day when channels were changing rapidly and every 3 to 4 days when there was little or no change. Table 6 summarizes the number of channels in the vicinity of each cross section and the estimated total discharge for White River at the time each panorama was photographed. In addition, figure 50 shows the number of channels at the odd numbered cross sections in relation to changes in discharge.

Both tranquil and shooting turbulent flow commonly occur in natural channels. Wolman and Brush (1961) have shown that meandering began with the onset of shooting flow in some of their flume channels in non-cohesive sand materials. They suggested that because

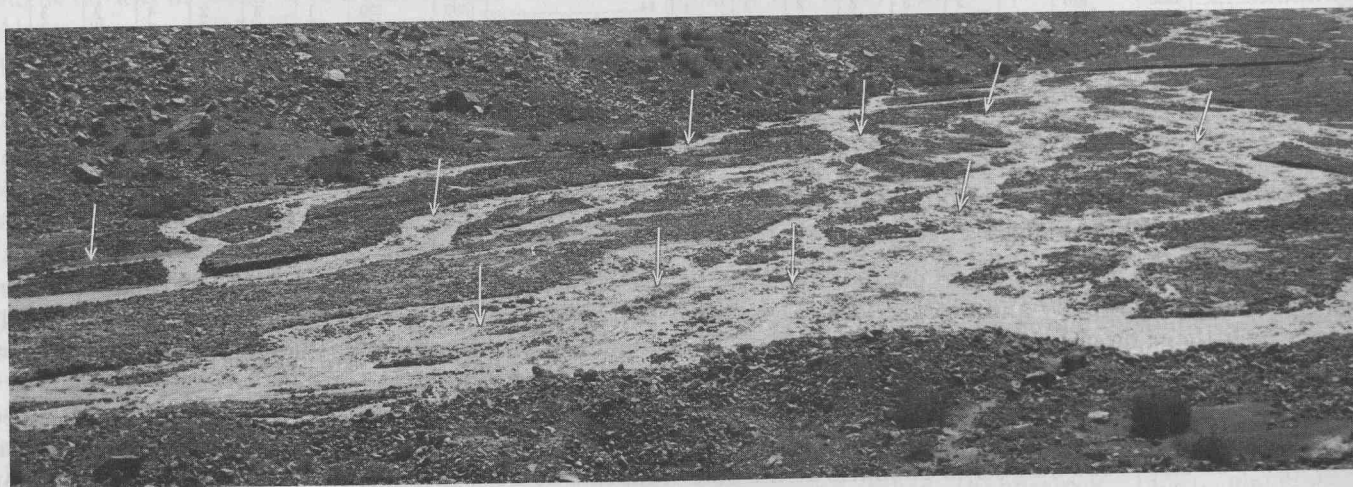
these meanders occur at much higher Froude numbers than those of major rivers they are not dynamically similar in all respects and, therefore, termed the former "pseudomeanders."

Photographs of White River taken after extended periods of low flow (less than 200 cfs) show what appears to be a meandering pattern from the junction of West and East Emmons Creeks to the gage. As the flows in these meanders have relatively high Froude numbers (table 7), they are not dynamically similar to common meanderings with much lower Froude numbers. The meanders of the White River shown in figures 41 to 43 may approach the condition of pseudomeanders in a natural channel.



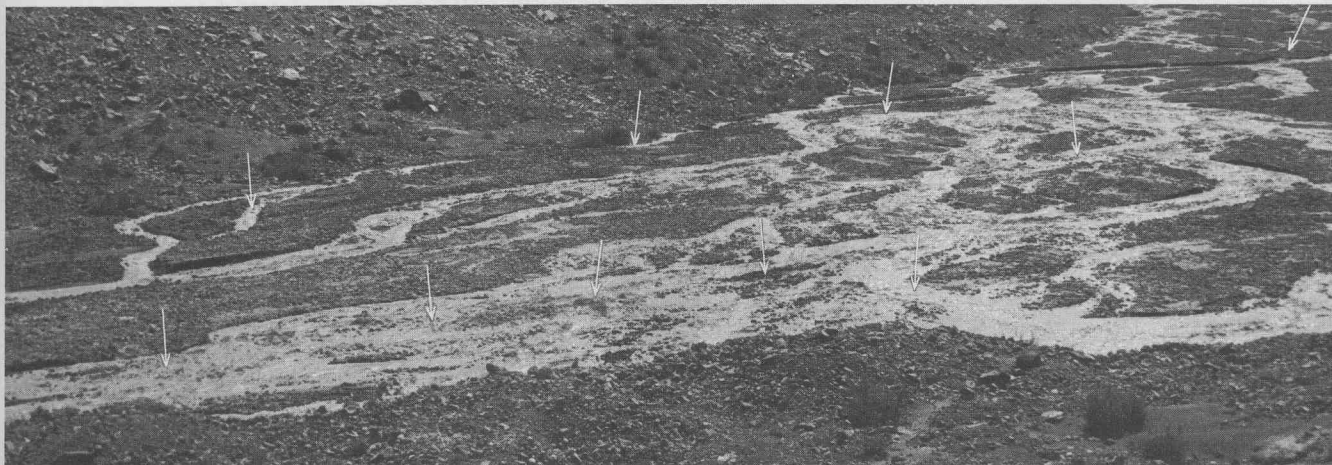


A. 1:00 p. m.



B. 1:15 p. m.

FIGURE 39.—Pattern changes, 1:00 to 2:00 p.m., July 18, 1958. A, 1:00 p.m.; B, 1:15 p.m.; C, 1:30 p.m.; D, 1:45 p.m.; E, 2:00 p.m. Discharge is about 350 cfs. Arrows indicate new channels or bars which have appeared since the previous photograph.



C. 1:30 p. m.



D. 1:45 p. m.



E. 2:00 p. m.





FIGURE 40.—Panorama of the White River valley train, July 18, 1958, at 10:50 a.m. Discharge about 340 cfs. Note kettle being filled in center foreground.



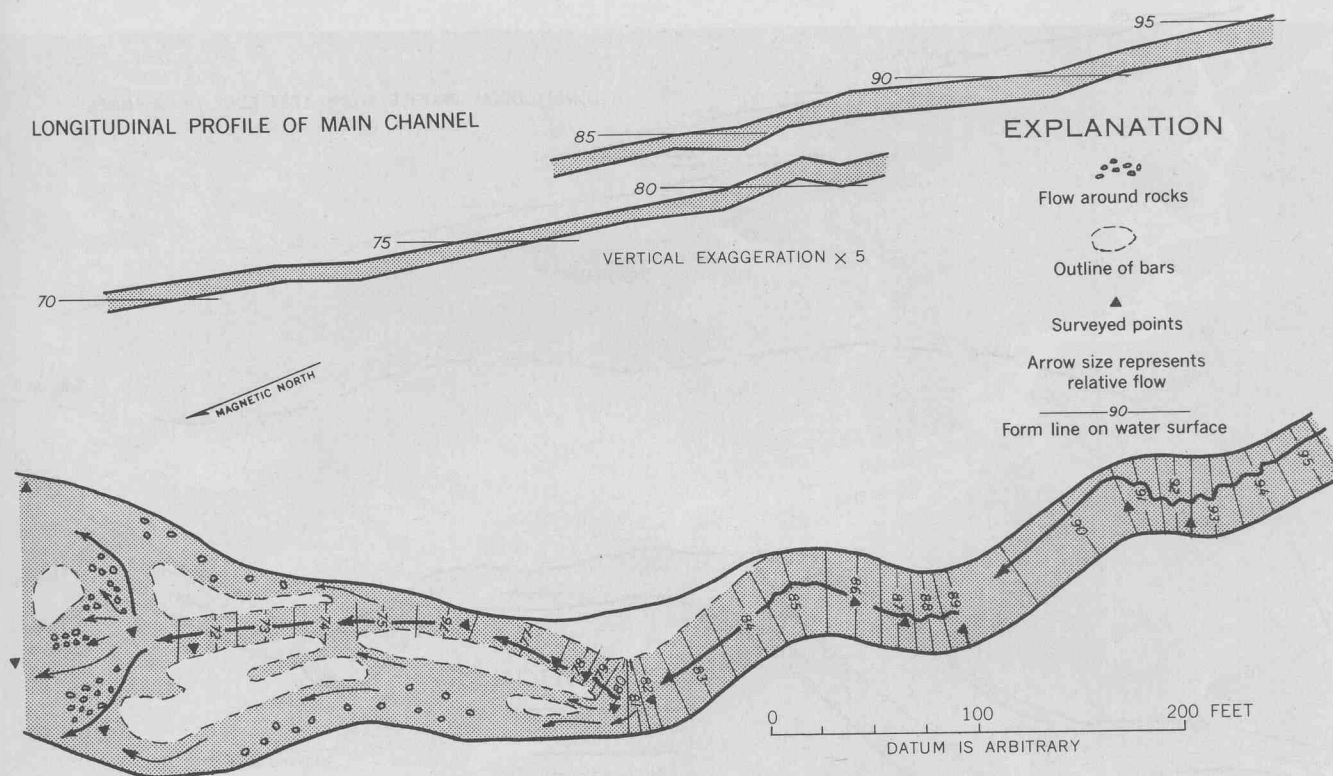


FIGURE 41.—Meanders and nose on the White River near cross section 5, August 13, 1958. Plane table map by Fahnestock and Bond.



FIGURE 42.—Meander pattern of the White River, September 30, 1958. Discharge is approximately 110 cfs. Photograph by John Savini.

During August 1957, East and West Emmons Creeks did not always join near cross section No. 2. At times East Emmons Creek flowed through an ice

tunnel along the right valley wall and joined West Emmons Creek through several channels, occupied at different times, which entered the valley train between Nos. 4 and 5. Braiding occurred more frequently below the junction than upstream from it.

When first visited in 1958 (June 18), there was a braided pattern with two or more channels (table 6; fig. 49) from the vicinity of cross section No. 5 to the constriction at the moraine. From the junction of East and West Emmons Creeks to No. 5 the flow was along the right side of the valley train in one channel, which received several small tributaries from the ice tunnel noted in 1957. By afternoon on the 18th there were more channels at all cross sections, and some flow had shifted to the left side of the valley train. By June 26, the flow upstream from No. 5 had shifted almost entirely from the right to the left side of the valley train; from Nos. 6 to 10 there were several channels; and farther downstream the flow had shifted to the right, concentrating in one channel. This situation remained, with minor changes, until about July 8, when the number of channels increased at all cross sections. During the next few days the flow was concentrated into fewer channels, and most channels were in the left half of the valley train between Nos. 5 and 7.

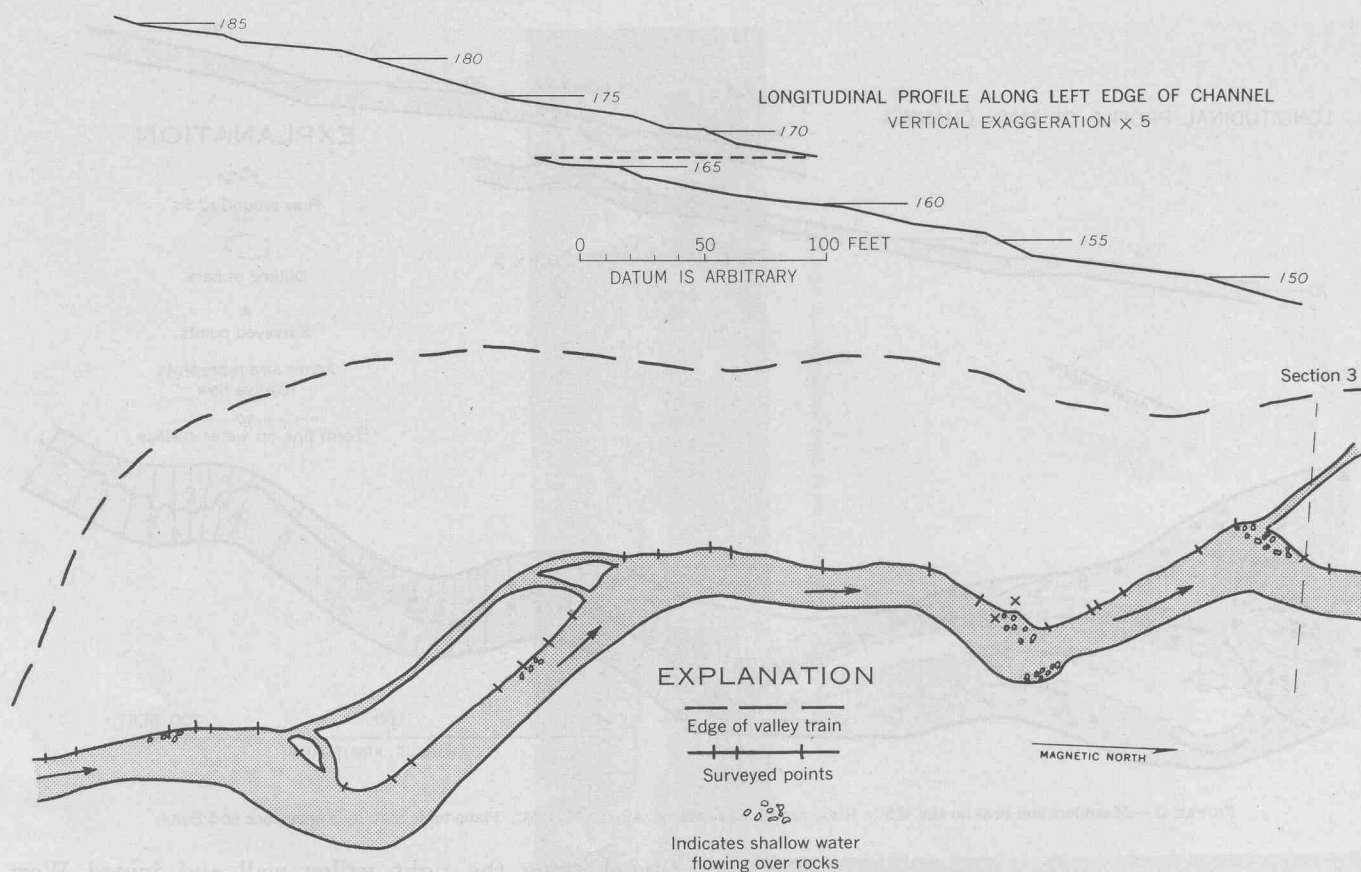


FIGURE 43.—Upper reach of the White River, August 13, 1959. Arbitrary datum. Planetable map by Fahnestock and Gott.

The number of channels at most cross sections continued to diminish, with minor shifts of position, until the 17th and 18th (figs. 39 and 40), when active braiding took place from cross sections Nos. 5 to 7, and limited braiding between Nos. 10 to 12. By July 21, this activity had decreased; likewise the number of channels had decreased at most cross sections with no apparent change on the surface of the valley from Nos. 2 to 4. The Emmons Creeks, after joining, had been diverted by their deposits into the ice tunnel on the right side of the valley train. The situation changed little except for shifts of the channels and braiding at Nos. 10 to 12. On July 28, the stream had left the ice tunnel, and active braiding occurred from the junction of the two creeks to the gage. The next day the number of channels had diminished (table 6). Between July 29 and August 3, the main channel between Nos. 5 and 7 was diverted from the center of the valley train to the right side and later back to the left. The pattern of August 4 is shown in figure 44. Only minor changes in pattern occurred until August 8 when active braiding took place between Nos. 5 and 7.

Photographs of the pattern on August 11 showed a general decrease in the number of channels at most cross sections. The main channel was along the left

side of the valley train from cross section No. 2 to the constriction above No. 10, where it changed to the right. This situation persisted until August 22, when active braiding took place from Nos. 5 to 11. From August 25 until early in September, the relict pattern of this braiding persisted between Nos. 5 and 7, whereas the main channel reverted to its right-bank position downstream from No. 7. Photographs taken by John Savini on a visit in late September (fig. 42) show that the stream had a meandering pattern.

The pattern on June 18, 1959, was similar to that of the previous September. The main channel was in approximately the same place, but the meandering was not as prominent and there were more channel islands. No major pattern changes took place until July 12, when braiding developed between cross sections Nos. 5 and 7. This braided pattern persisted with minor changes until July 17, when there was renewed braiding in the same reach. No channels were present on the right half of the valley train from No. 3 to the constriction above No. 10, where the stream crossed over to the right side. A small clear stream issued from kettles near No. 10 and flowed down the left side of the valley train to the main channel at No. 12. The active braiding of July 17 continued on



FIGURE 44.—Relict braided pattern of August 4, 1958. Discharge is approximately 220 cfs. Note stream through ice tunnel.

the 18th, and pattern changes were rapid between Nos. 10 to 12 on the 19th. Fig. 45 shows changes between July 19 and 21 in channels near No. 10. The braided pattern persisted with shifting channels and bar development in the reach between Nos. 5 and 7, and on July 24 vigorous braiding took place from No. 3 to the gage, although no streams were present on the right half of the valley train above No. 10. This pattern persisted with minor modifications and a decrease in the number of channels until the middle of August, when the gradual development of a meandering pattern became apparent, although a number of islands remained. This meander pattern continued to develop into September. October, November, and December storms may have caused the stream to revert temporarily to a braided pattern after each storm.

#### FEATURES OF THE WHITE RIVER CHANNEL PATTERNS

Features of the White River patterns are shown in figures 41 and 46 to 48. Figure 46 shows the braided pattern and water-surface profile near cross section 7. Inspection of the panoramas suggests the flows of August 20–25 were probably responsible. For a detailed map of the feature called topographic nose,

see figure 47 (August 26). On that date the nose, which had already developed, was gradually being modified. The depths of water are shown by means of bottom and water-surface contours. Such features appear to be a type of small alluvial fan and are quite common on the valley train. Other examples are shown in figures 48 and 41. In each example there is a relatively deep and swift reach upstream which is confined by either cutbanks or "levees." At the end of this reach, the water spreads and flows in several directions in sheets or in a number of poorly defined channels. When the feature first develops, boulders rolled through the swift reach are deposited with a great amount of noise and splashing on the bars at the end of the chute. Often the deposition of these bars blocks other channels (right foreground, fig. 39), forming pools. A delta is then built into the pool from the main stream and from the blocked stream with fine material settling out in the deeper and quieter portions of the pool. These deposits are the only concentrations of fine materials on the valley train.

The subsequent history of the nose depends on the amount of water and sediment delivered to it. Often a decrease in load occurs at some time after deposition of the nose with a consequent channeling of the deposits



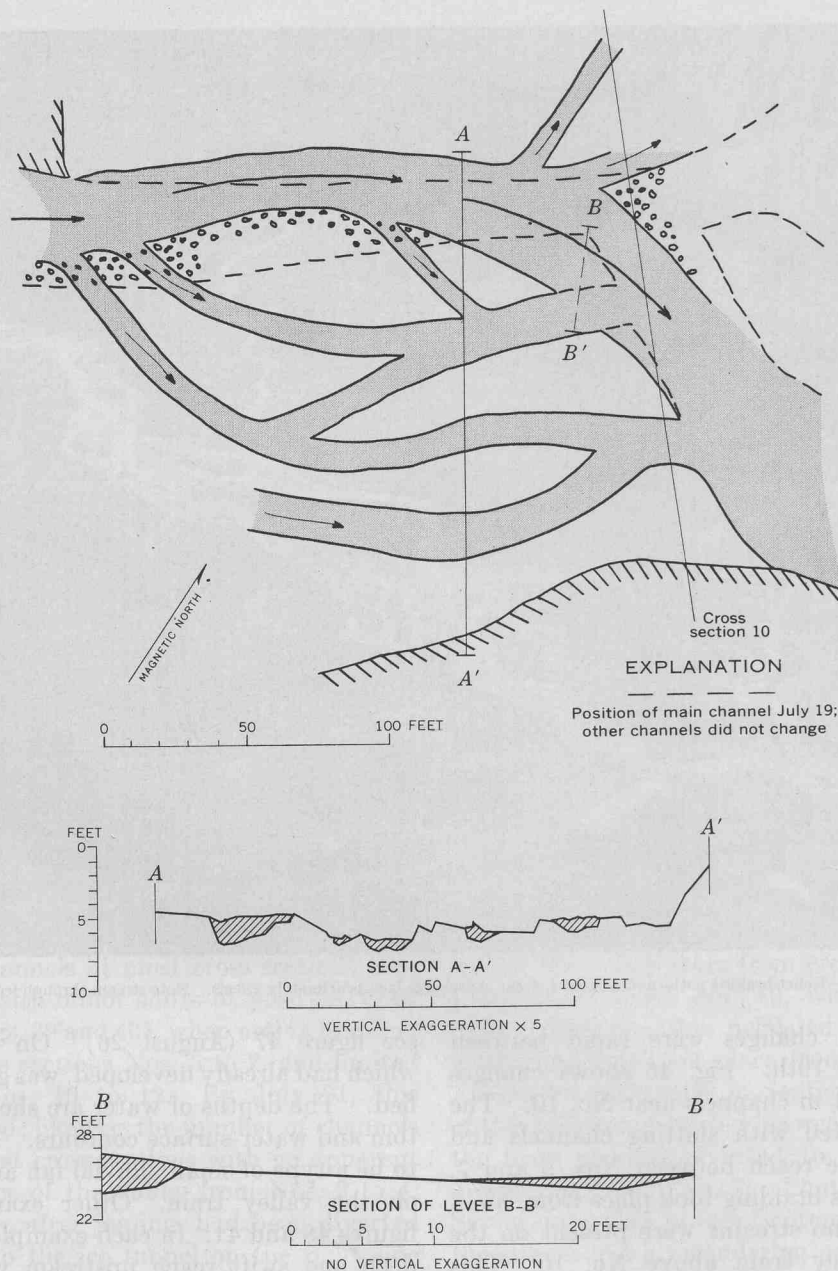


FIGURE 45.—White River near cross section 10, July 21, 1959. Planetable map by Fahnestock and Gott.

and concentration of flow. If sufficient material is available as a result of this scour, the nose may then be left in the form of cut banks and levees and a second nose deposited downstream.

The term "nose" has been used here, as these features actually project above the general slope of the valley train. It is possible to sit on the streambank near one of these features and watch the water rolling boulders at eye level only a few feet away across the stream. These noses or alluvial fans may be similar in some respects to the horseshoe bars described by Hjulström (1952, p. 340).

Contrary to what might be considered normal behavior for a stream, White River channels are often not in depressions but along the top of a ridge. This ridge is formed by the natural levees like those shown in figures 45 and 48. Striking changes in alignment and pattern are caused by deposition of levees that shift the position of the channel, confine it, or block off a channel entrance, thus diverting the flow to another channel as if a valve had been closed. These levees may be similar to the features described by F. E. Matthes (1930, p. 109). Levees may be formed also by the coalescing of bars in a bed of the river, confining

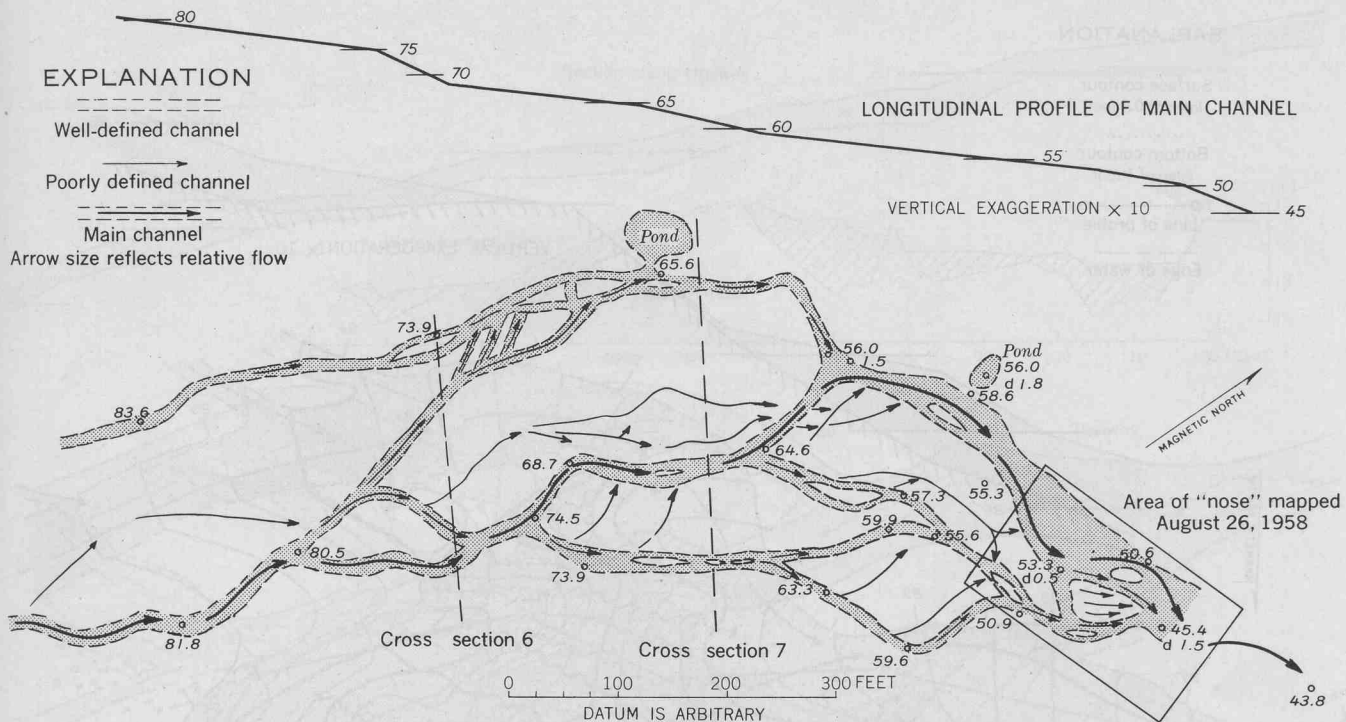


FIGURE 46.—Relict braided pattern of the White River, September 3, 1958. Planetable map by Fahnestock and Bond.

the flow to a narrower and deeper channel. These bars appear to be similar to those described by Hjulström (1935, p. 340). Hjulström pointed out that the thalweg of a glacial stream is not necessarily on the outside of these bends, as in a normal channel, but may occur in the middle or on the inside of the bend. Bars are formed quite rapidly under favorable conditions (fig. 39). Time-lapse photography is quite useful in recording and analyzing the changes which take place.

Figures 41 to 43 show reaches of meandering channel. Water-surface slopes are shown for both figures 41 and 43, and the depth of water is shown along the thalweg in figure 41. It is interesting to note the common riffles or crossovers and pools are missing but there are periodic changes in slope which correspond to bends in the plan view. The water surface shows alternations of steep and gentle slopes as does the channel bottom. The gentle slopes are characterized by swift, relatively smooth flow, whereas that in the steeper part appears to be much more turbulent and the bottom much rougher. The fact that along the thalweg there appears to be little if any shallowing in the steeper reaches may well be misleading, as the mean depth may be less because of the greater irregularity of the bed in these sections.

The "meanders" have the peculiar appearance of a series of bends connected by short straight reaches with swift quiet flow. The steeper parts are usually associated with bends. From the sketch maps (figures

41, 43, 45, 46 and 48), it is apparent that no greater sinuosity is associated with the meanders than with the anastomosing channels. As Leopold and Wolman (1957) observed in several rivers, individual anabranches may meander, and meandering and braiding reaches may alternate along the stream channel.

#### ANALYSIS OF PATTERN

##### CHANNELS—NUMBER, PERSISTENCE, AND LOCATION ON THE VALLEY TRAIN

Figure 49 shows the number of channels at each cross section for each discharge range. The complexity of the problem of determining the cause of braiding and variation in number of channels is well illustrated by this figure. The direct relation between the number of channels and discharge at each cross section is qualified by the number of times that a particular discharge has occurred, probably because the more frequently a discharge occurs the more likely it is to occur at least once under optimum conditions for braiding, and because a lower discharge tends for a time at least to occupy most if not all of the channels of the preceding high flow. It is also qualified, as shown by figure 50, by the time of occurrence and duration of the particular discharge. If a lower discharge occurs for several consecutive days following a higher discharge, the number of channels gradually diminishes.

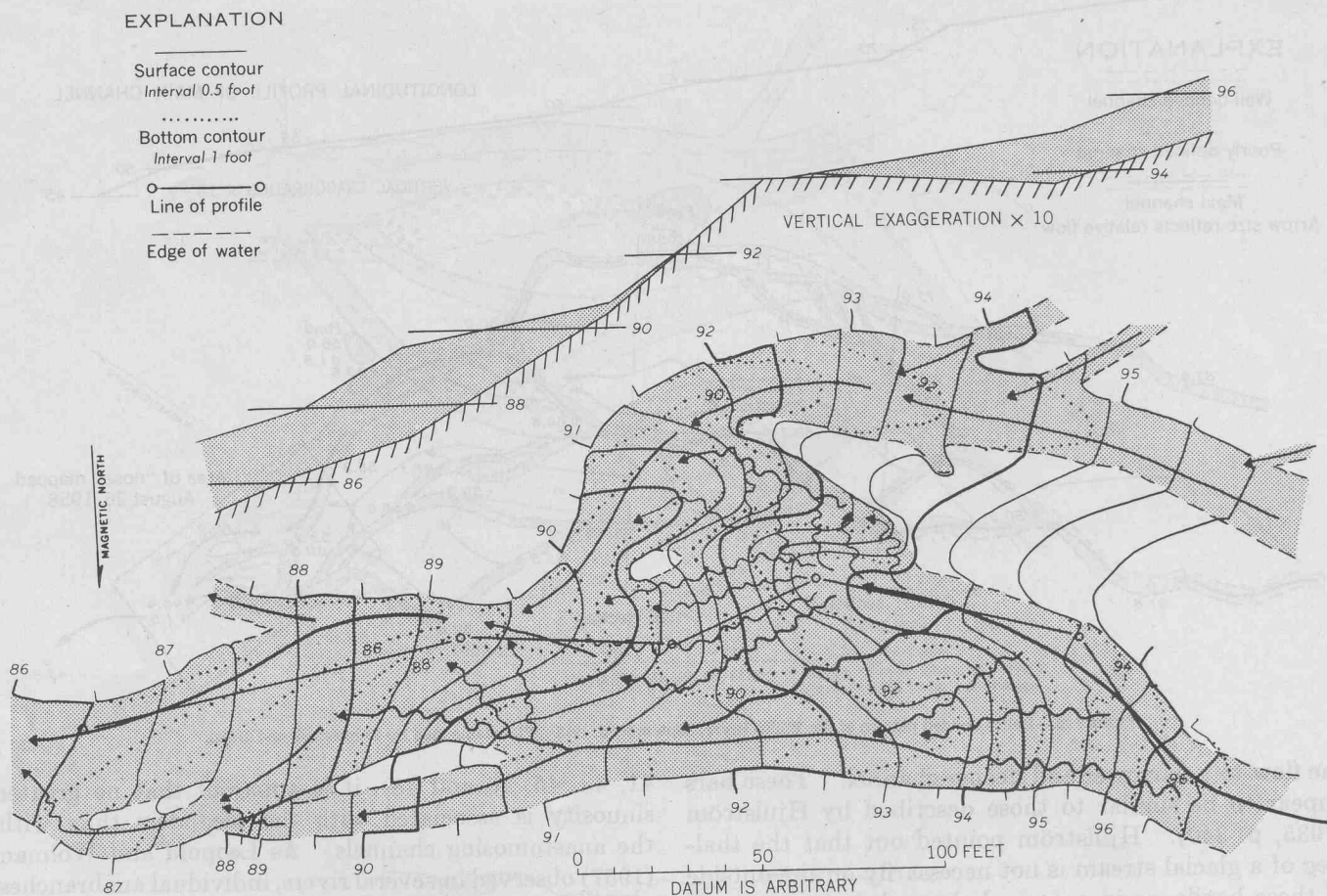


FIGURE 47.—White River nose. August 26, 1958. For location see figure 46. Planimetric map by Fahnestock and Bond.

The best correlation in figure 49 is that of number of channels with location on the valley train. This correlation appears to be in part a function of the width of the valley train at that point. The factor of slope must be considered, but its role is obscured by the decrease in number of channels below the constriction between cross sections 7 and 10. One might surmise that optimum conditions exist at cross sections 6 and 7 from the large number of channels. These cross sections have valley slopes of 0.04. This large number of channels occurs less frequently at the cross sections both upstream and downstream from Nos. 5 and 6, which are on higher and lower valley slopes. It is thought that slope may be a dominant factor and that the deposition which took place on Nos. 3 through 7 in 1958 may account for most of the material brought into this reach by the stream. This deposition deprived the stream of a heavy bed load farther downstream and lessened the likelihood of its braiding at Nos. 10, 11 and 12.

An alternative explanation to account for the number of bifurcations in a given reach emphasizes the importance of valley train width in conjunction with the distance below the glacier and distance between cross sec-

tions. Two cross sections only a few feet apart are likely to have the same number of channels at the same time, and the number of chances a stream has to bifurcate is directly proportional to the length of reach and the number of channels within the reach. A channel along the valley wall is limited in its freedom to divide or migrate in the direction of the valley wall. This control operates where the valley wall is unconsolidated, by heavily loading the impinging stream, and causing the deposition of a bar which diverts the flow from the wall. Thus, the longer the cross section and the farther it is downstream from a constriction, the greater the freedom to braid.

Constrictions exist at the junction of the East and West Emmons Creeks and between cross sections Nos. 7 and 10. Although from these considerations Nos. 3 and 11 should have a similar number of channels, No. 11 should have more because there are usually more channels upstream from the constriction. Fewer channels at No. 3 than at No. 11 might also be favored by the higher valley slope (0.075 ft per ft) as the competency and capacity of the stream are likely to be greater than on the lower slopes at No. 11. Inspection



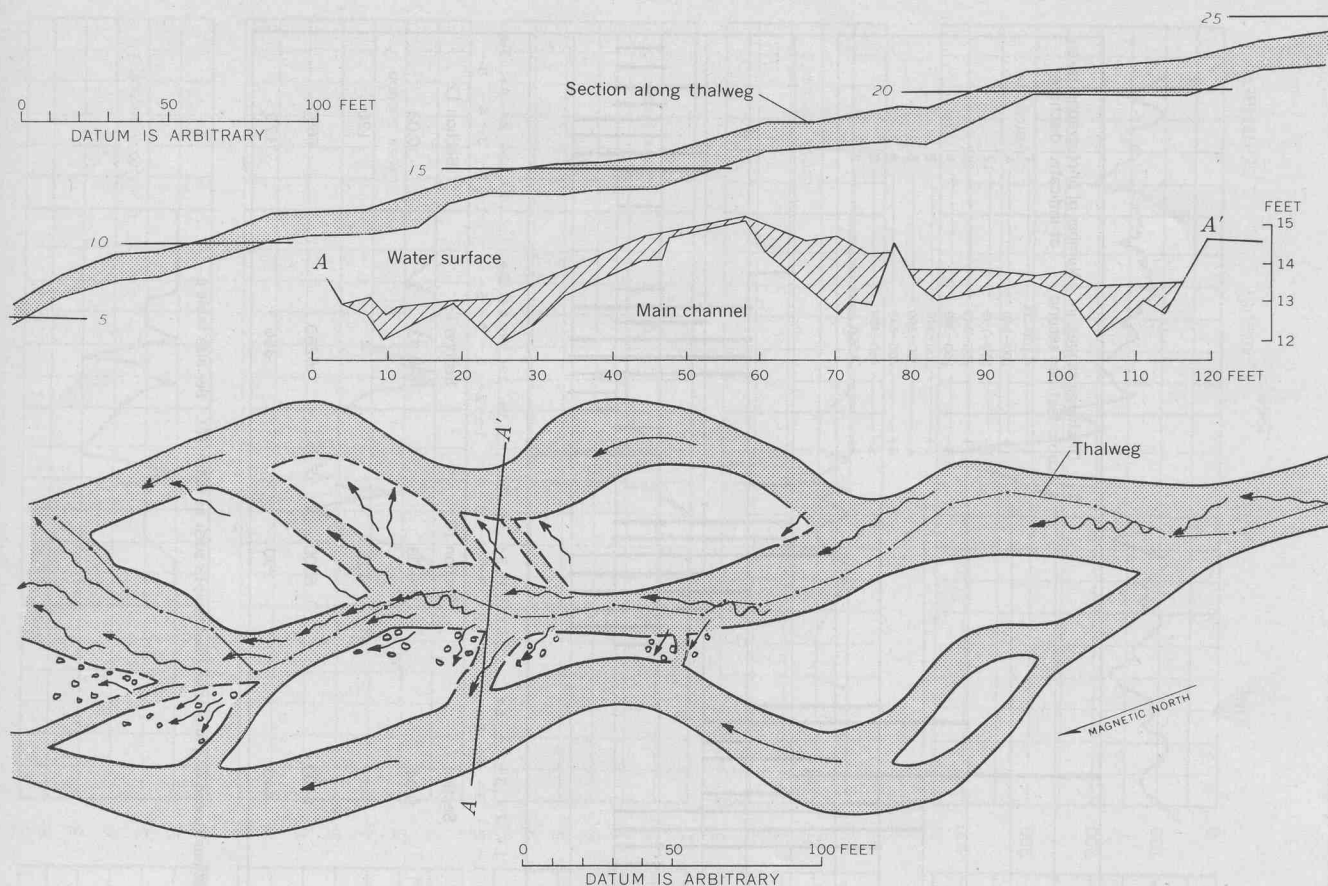


FIGURE 48.—A braided reach of the White River between cross sections 5 and 6. Longitudinal cross section of thalweg and cross section of channel surveyed August 8. Planetable map, August 7, 1959, by Fahnestock and Gott.

of figure 50 and table 6 verifies the number of channels, for the two cross sections are similar, No. 11 having slightly more channels than No. 3. The relatively large number of channels at No. 10 is due to at least in part to the persistence downstream of some of the many channels at No. 7 in spite of the effect of the constriction.

#### RELATION OF PATTERN TO CHANNEL CHARACTERISTICS

It appears that channel pattern and the hydraulic characteristics of the channels should be related. Leopold and Wolman (1957, p. 63) found that—

When streams of different patterns are considered in terms of hydraulic variables, braided patterns seem to be differentiated from meandering ones by certain combinations of slope, discharge, and width-to-depth ratio. Straight channels, however, have less diagnostic combinations of these variables. The regular spacing and alternation of shallows and deeps is characteristic, however, of all three patterns.

Their plot of the relation between slope and discharge (Leopold and Wolman, 1957, figure 46, p. 59) for the three types of channels—braided, meandering, and straight—showed that the line  $s=0.06 Q^{0.44}$  divided meandering channels from braided ones for the streams that they studied. A braided pattern occurred on

higher slopes for a given discharge and at a higher discharge for a given slope than did a meandering pattern. No clear-cut relation was found for “straight” channels although most of them lay above the line.

It was hoped that a study of the White River channels might aid in explaining the mechanism which produced this separation. A similar plot of the White River channels for which slope data were available showed, however, no such systematic division. All White River channels lay above the line in the braid region. The “pseudomeanders” of the White River occur on slopes and at discharges similar to those of the other patterns. The description of these “meanders” as straight channels joined by bends may be quite appropriate. An increase in discharge did produce a pronounced tendency to braid as is shown by the seasonal change from meandering to braiding (figs. 40, 42).

Leopold and Wolman (1957, p. 53) described the formation of a braided pattern in this way:

A mode of formation of a braided channel was demonstrated by a small stream in the laboratory. The braided pattern developed after deposition of an initial central bar. The bar consisted of coarse particles, which could not be transported under local

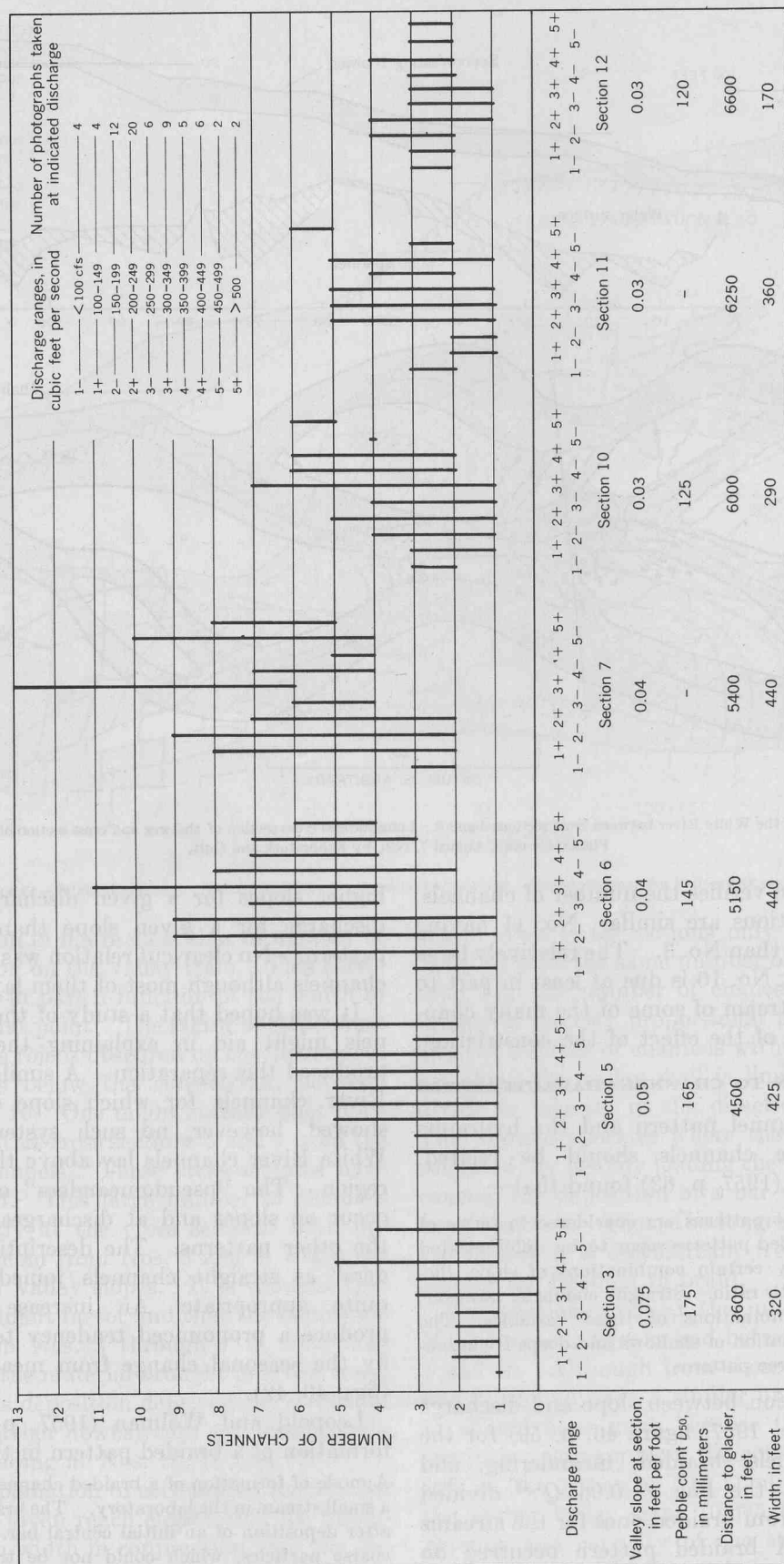


FIGURE 49.—Relation of discharge range to number of channels at cross sections (based on panoramic photographs such as figure 40). See also table 6.

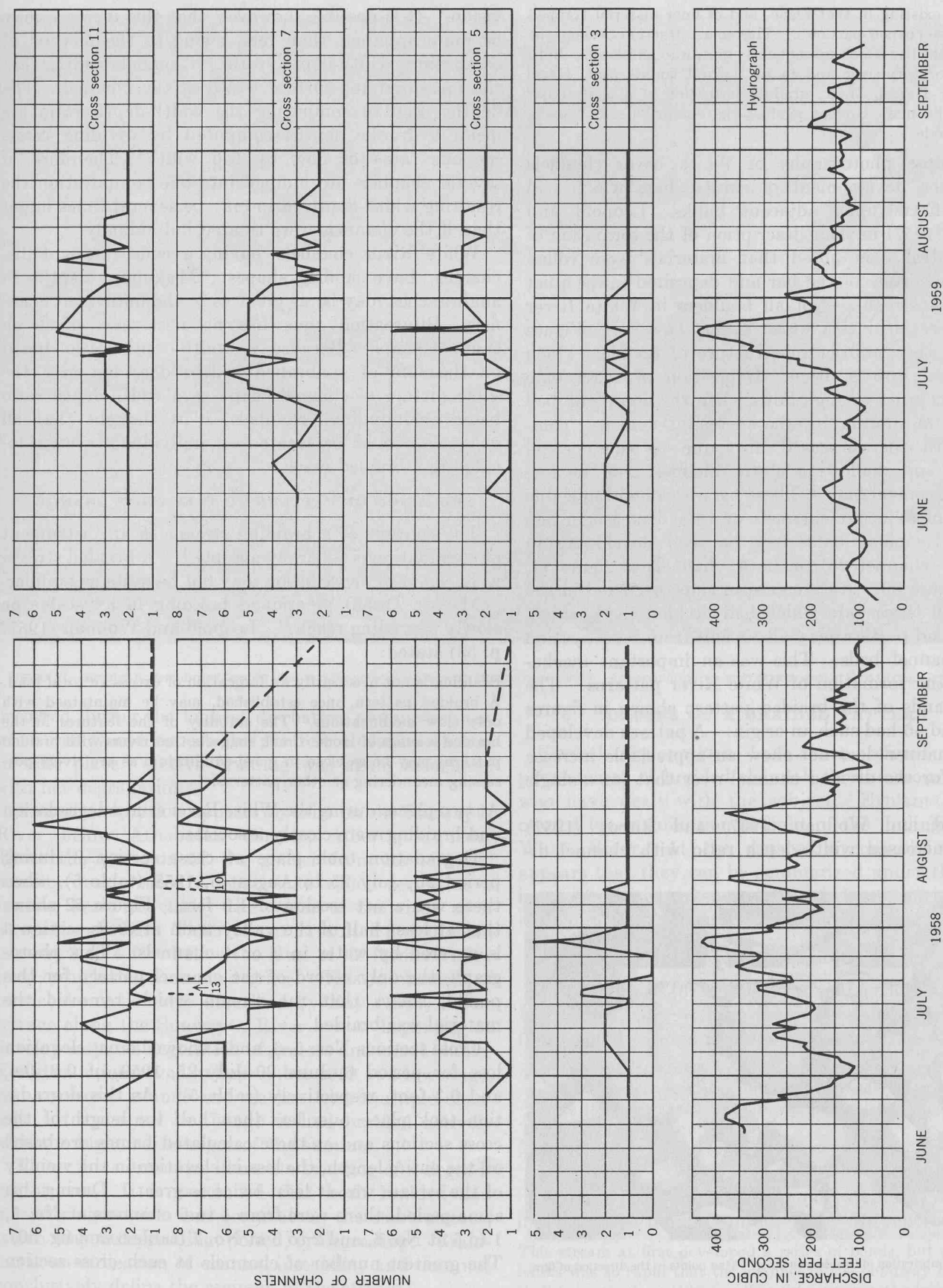


FIGURE 50.—Hydrograph of discharge and number of channels at selected cross sections.



conditions existing in that reach, and of finer material trapped among these coarser particles. This coarse fraction became the nucleus of the bar which subsequently grew into an island. Both in the laboratory-river and in its natural counterpart, Horse Creek near Daniel, Wyo., gradual formation of a central bar deflected the main current against the channel banks causing them to erode.

Time-lapse photography of White River channels showed the development of similar bars which had similar effects upon adjacent banks. Leopold and Wolman (1957) in their description of the formation of these central bars noted that materials were rolled across the surface of the bar and deposited in the quiet water downstream. Not all boulders in White River channels reached this quiet water, however. Figure 51 shows the "imbricate" structure of boulders which were rolled into position. Deposition of these bars usually occurred at high flows when the abundant bed load, set in motion by the increased velocity, combined with the increased discharge to cause rapid widening and shallowing of the channel and the formation of central bars. These bars became islands due to scour of adjacent channels or to a decrease in discharge. Incipient bars may be seen developing in figure 10 (channel 26), in both channels of figure 11, and in figure 39. An increase in slope occurred in the vicinity of these bars which had divided the channel.

A braided pattern may also result from reoccupation of old channel beds. This was an important mechanism in the formation of White River patterns. The major islands of the braided pattern shown in figures 39, 44, and 46 had such an origin. A pattern developed in this manner does not show an appreciable increase in slope for the divided channel over that for a single channel.

Leopold and Wolman (1957) and Rubey (1952) cite an increased width-depth ratio with channel di-

vision. It is possible, however, that this increase may be more apparent than real, owing to the method of computing width-depth ratio. Channel widths are the measured top-surface width of the channel. The depths used in computing the width-depth ratio are mean hydraulic depths computed by dividing cross-sectional area of flow by top width. Therefore, if several channels are lumped into one computation the resulting width-depth ratio may be several times larger than if the channels were treated individually.

White River channels, having a wide range of discharges, have similar shapes. Maximum depths in anabranches may be as great as in the undivided channels. Bifurcation thus has no necessary effect on channel shape. Because a width-depth ratio based on the sum of anabranch widths does not give the same picture of channel shape as a width-depth ratio based on individual channels, it is thought that all channels should be treated as individuals even if located in a braided reach.

#### RELATION OF PATTERN TO ELEVATION CHANGE

The concept of a braiding stream as an instrument of aggradation is generally accepted. A braided stream as an agent of degradation may not be quite as familiar.

Mackin (1956) mentioned braiding in a "stable or slowly degrading reach." Leopold and Wolman (1957, p. 53) stated:

Braiding is not necessarily an indication of excessive total load. A braided pattern, once established, may be maintained with only slow modifications. The stability of the features in the braided reaches of Horse Creek suggests that rivers with braided patterns may be as close to quasi-equilibrium as are rivers possessing meandering or other patterns.

At two places during the White River study degradation and braiding were closely associated.

Degradation took place at cross section 3 during period 2b, July 25 to August 5, 1958 (table 5), when there was a net erosion of 1.9 feet. Figure 52 shows that at least half of the valley train at cross section 3 is covered by water in 3 or 4 channels. This photograph, the only record of the channel pattern for this period, shows that the stream which removed the material was braided.

Cross sections Nos. 5, 6, and 7 showed a net elevation loss for period 6, June 20-July 25, 1959, of 0.2, 0.4, and 0.3 foot, respectively (table 5). As this degradation took place over less than half the length of the cross sections and as these calculated values are based on the entire length, the loss in elevation in the vicinity of the stream was at least twice as great. During this same period, there were from 1 to 6 channels at No. 5, 1 to 5 at No. 6, and 2 to 6 at No. 7 (table 6 and fig. 50). The greatest number of channels at each cross section



FIGURE 51.—Imbrication of boulders on a bar. Man points in the direction of flow.



FIGURE 52.—Braided pattern during the degradation of the valley train between cross sections 2 and 4. Arrows indicate the ends of cross section 3. 4:00 p.m., July 28, 1958.

occurred during the highest discharges of the summer, when one would expect the greatest elevation change to take place. Active braiding was again associated with net degradation.

A similar situation appears to exist on the Sunwapta River below Athabaska Glacier, Alberta, Canada. There the recession of the glacier from its terminal moraine has created a small lake which serves as a settling basin to clarify the water of the river as it leaves the glacier. This river, as a result of lack of load, has cut 3 to 4 feet below the level of its former valley train, which now forms a terrace. This mechanism of terrace formation was described by Ray (1935). The terrace along the Sunwapta River is now beginning to develop a sparse vegetation of grasses and small plants. The stream, observed in September 1959 at a low stage, had numerous islands and reaches with multiple channels. It appeared as though it had braided actively with the higher discharges of the summer.

These three examples illustrate braiding of a degrading stream. It is concluded that both braided and meandering reaches can occur along the same stream, which may be aggrading, poised, or degrading. Braiding is an indication of channel instability and does not conclusively define the regimen of the stream.

#### CAUSES OF A BRAIDED PATTERN

Several explanations have been offered for the phenomenon of braided channels by numerous authors who have dealt with the subject. Explanations are quoted for examination in light of the findings on the White River. At the risk of oversimplification it appears that they can be summarized under the headings: erodible banks, rapid and large variation in discharge, slope, abundant load, and local incompetence.

##### ERODIBLE BANKS

Fisk (1943, p. 46) suggested that the character of the Mississippi River reflects that material through which it flows. He stated that the Mississippi has—

1. A tendency to braid where bank caving is active. Bank caving is active where sediments are easily erodible. Sands are the most easily erodible sediments, less permeable sediments are tougher.
2. Tendency to braid where slope is steep and sediments easily erodible, and where slope is excessively low and load great.

Friedkin (1945, p. 16) in his model studies of meandering, produced braiding in one test. He reported:

This stream at first developed a series of bends, but erosion of banks was so rapid that the channel in the bendway became as shallow as the [point] bars. The flow overran the bars and dis-



persed through the channel . . . No sand was fed at the entrance of this stream; braiding resulted solely from excessive bank erosion. As the channel shoaled and the bed of the river raised, the slope became steeper.

Mackin (1956) in describing the braiding of the Wood River of Idaho stated that the river

. . . meanders in a forest for many miles, braids in a 3-mile segment where the valley floor is prairie, and returns to a meandering habit where the river re-enters a forest. The river is stable or slowly degrading in all three segments. The essential cause of the drastic difference in channel characteristics in adjoining segments is a difference in bank resistance due to presence or absence of bank vegetation.

Much of the material in transport by the White River is probably derived from bank erosion; but this factor alone, although it may occasionally develop a braided pattern, is insufficient to cause the active braiding of White River channels. Frequently at the same time and discharge, both braided and meandering reaches were present on the valley train. At times the number of channels was increasing in one area while decreasing in another. Braiding and meandering were not limited to specific parts of the valley train. All such changes on the White River cannot be explained in terms of changing erodibility of banks but must reflect other factors that are more easily changed. It is possible that winnowing by the high flows of 1958 may have slightly decreased the erodibility of the reach of the White River between cross sections 2 and 4, giving it a stability in 1959 relative to 1958. A more probable explanation for this relative "stability" is that in 1959 less bed load was transported through this reach, affording less opportunity for deposition within the channel.

#### RAPID VARIATION IN DISCHARGE

Doeglas (1951) stated that none of the frequently mentioned causes of braiding ("greater slope, loose debris, or greater discharge") accounted for the pattern of some rivers during the Pleistocene. He suggested (p. 297):

Large and sudden variations in the runoff seem to form braided rivers. A more regular runoff throughout the year gives a meandering river. The gradient and the available sedimentary material seem to be of little importance.

The White River does show a tendency toward adjustment to higher discharges where they are continued for a period of several days. In both 1958 and 1959, a discharge sufficient to produce radical changes in pattern at some profiles during June and early July produced relatively minor changes in August; for example, the number of channels at cross sections 3 and 5 during August 1958 varied from zero to 7 (fig. 50). The change in discharge and previous flow history appear to be almost as important as quantity of water

in producing changes in pattern. This may be an illustration of Doeglas' idea of discharge variation as a cause of braiding. It is a likely mechanism for deriving more load for the same discharge and might work as follows in a White River channel. During period of low flow, the fine sediments would be winnowed from the bed of the channel in some places and deposited in others, but coarser sediments would be left on the bed. With a significant increase in discharge, these coarser sediments would start to move and new supplies of finer materials would be uncovered. The increased discharge might also reoccupy abandoned channels and set in motion all the materials deposited in them by waning stages of the former stream. This excess of load would cause local deposition, which in turn would result in scour of adjacent banks and an increase in load. A more gradual increase in flow might allow the stream to develop a channel that could transport the increased load without forming bars and developing a braided pattern.

The frequency with which braided reaches are interspersed with meandering reaches (Leopold and Wolman, 1957, and Mackin, 1956) and laboratory studies (Friedkin, 1945; and Leopold and Wolman, 1957) during which braiding was produced with no variation in discharge would seem to indicate that rapid discharge variation is eliminated as a cause for braiding in most streams.

#### SLOPE

The suggestion that change in slope alone is sufficient to cause braiding does not explain phenomena observed along the White River. Braiding developed on slopes which range from 0.01 to 0.20, but only coincident with bed-load movement of coarse materials. The river frequently braids in one part of the valley train on slopes both higher and lower than slopes of other parts where it has only one channel. In some cases, slope may serve to aid the stream in setting in motion enough material to form a braided pattern. In others, it may serve only to maintain velocities so high that the deposition of bars does not take place.

#### ABUNDANT LOAD

Hjulström (1952, p. 310) stated:

A fundamental fact for understanding the braiding of rivers is the great sedimentary load which they carry.

Russell (1939, p. 1200, 1201) stated:

Available load seems to be the factor separating meandering from braided streams. A smaller load, in proportion to carrying capacity, at the moment, makes for meandering, a larger load for braiding \* \* \* Those [streams] flowing through sandy or gravelly flood plains are more readily overloaded and therefore tend to anastomose, or display the effects of braiding.



Rubey (1952) in his report on the Hardin and Brussels quadrangles in Illinois suggested that the form of the Mississippi and Illinois Rivers in that area resembled a braided stream more closely than a meandering one, stating (p. 123)—

\* \* \* they follow somewhat crooked courses, it is true, but the curves are due not to meander growth but to diversion of the channel by large alluvial islands.

He noted (p. 124)—

It is probably a significant fact that the islands are larger and more numerous at the mouths of tributary streams \* \* \* An alternative interpretation explains most of the islands much more satisfactorily. Tributary streams commonly have steeper gradients and carry more debris per unit volume of water than the river; hence they deposit part of their load as deltas and as submerged bars across the tributary mouths. Some of the sand bars and flats built at times of flood stand well above the water level at low and normal stages of the river \* \* \* some of the willow bars grow larger and higher until they become "timber islands" the surface of which is built to the level of the mainland flood plain and covered with dense growths of large hardwood trees. Once an island reaches this stage it becomes an essentially permanent feature.

The pattern changes of the White River in 1958, which were both more frequent and more widespread than those in 1959, illustrate the role that abundant bed load played when it was introduced to the section of valley train under observation. That the material was not derived by bank erosion within the area is evident from the net gain in elevation for the valley train (1.2 ft for the year, table 5). In 1959 the decrease in activity and relatively little braiding in the reach between cross sections 2 and 5 suggest that the braiding that did take place was largely caused by bed load derived within the reaches under observation. The net loss of elevation on cross sections 5 to 7 during the summer of 1959 suggests that material was derived from both bed and banks and that braiding occurred in a degrading reach of the stream.

To the writer's knowledge, it has not been suggested that a braided channel pattern can be developed without an appreciable bed load. The common element in all the above explanations appears to be a movement of bed load exceeding local competence or capacity, and consequent deposition within the channel causing the diversion of flow from one channel into one or more other channels. Observations of braiding by the White River suggest that the rate of change of the pattern in a stream which is not restricted by resistant banks is controlled by the amount of bed load. From observations of White River and the suggestions of authors cited, braiding is favored by the presence of erodible banks and fluctuations in discharge.

## RELATED PROBLEMS

### LONGITUDINAL PROFILE OF VALLEY AND EQUILIBRIUM IN A REGRADING GLACIAL STREAM

Many similarities exist between the White River as it deposits and reworks the materials of the valley train and the graded streams described by Gilbert (1877), Davis (1902), Kesseli (1941), Mackin (1948) and others. Mackin (1948, p. 471) defined a graded stream as—

\* \* \* one in which, over a period of years, slope is delicately adjusted to provide, with available discharge and with prevailing channel characteristics, just the velocity required for the transportation of the load supplied from the drainage basin. The graded stream is a system in equilibrium; its diagnostic characteristic is that any change in any of the controlling factors will cause a displacement of the equilibrium in a direction that will tend to absorb the effect of the change.

He emphasized the idea of a system in long-term equilibrium while recognizing similar short-term changes in the regimen. Rubey (1952), Leopold and Maddock (1953), and Wolman (1955) have discussed the equilibrium adjustment of the channel cross section. Wolman (1955, p. 41) applied the term "quasi-equilibrium" to adjustments made by streams that were not related to the length of time a set of conditions exists. This term is perhaps unnecessary, as the diagnostic characteristic of a graded stream cited by Mackin above is just this tendency to adjust.

Mackin (1948, p. 477) recognized the similarities between streams that were raising the level of their beds and graded streams and suggested (p. 478) that the term "aggrading" be restricted to upbuilding at grade. The White River, during the period of study, would appear to fit this definition in the reach from the junction of the Emmons Creeks to the gage. As it appears to be cutting down upstream from the junction, it then could be described as a regrading stream showing adjustment between the variables of slope, channel, cross section, discharge, bed material size, and load.

### INFLUENCE OF GLACIER REGIMEN ON THE DEVELOPMENT OF A VALLEY TRAIN

Emmons Glacier has retreated about 1 mile since 1910, most of this distance since 1930, and has advanced about 0.1 mile recently as shown in figure 3. The stream in 1958 and 1959 has deposited upon and eroded the valley train, resulting in net deposition in 1958 and net erosion in 1959. This response of the stream is probably related to the difference in weather and availability of material in the two years. It is apparent that the response is not related to the glacial advance of approximately 100 feet during this period. Observations of the stream, at the point at which it issues

from the glacier and downstream, suggest that most of its load is derived by erosion downstream from the glacier. The effect of the stream on the valley train would then seem to be a function of the amount of water provided by glacial melt rather than a function of the load introduced directly to the stream from the glacier.

The glacial regimen would seem to be related to the valley-train deposits only through the quantity of water that it makes available to the stream and the debris that it has deposited, subject to erosion by the stream. For example, the melting back of Emmons Glacier has uncovered large quantities of debris which are now available to the stream. Continued advance of the Emmons Glacier may radically alter the availability of this material and thus cause changes downstream. Similarly, changes in position of the termini of continental glaciers of the past must have radically altered the availability of debris to their drainage streams. Such effects are probably restricted to the immediate vicinity of the glacier, as a stream on alluvium will derive a load within a short distance and this load will be determined by its discharge, gradient, and availability of readily erodible material.

If periods of glacial advance and retreat can be considered analogous to winter and summer on the White River, advance (winter) would provide much smaller discharges and less debris; and retreat (summer) larger discharges and more debris. This is obviously a great oversimplification, as there were winters and summers, in the glacial climate. An additional assumption would be a constant rate of glacier flow because fluctuation in the glacier flow rate might change the position of the terminus without changing the discharge. The removal of a continental glacier from a drainage basin could well bring a decrease in the discharge of the stream, bringing to a close the deposition of a valley train.

While the regimen of Emmons Glacier has long-term effects in providing debris to the stream, the short-term effects of weather and runoff determine the rate of deposition and erosion, the hydraulic characteristics, and the pattern of the stream.

#### SUMMARY

1. Channels of the White River, developed in coarse noncohesive materials, are narrower, slightly shallower, and have higher velocities of flow on higher slopes at discharges of similar magnitude than channels with cohesive bank materials as exemplified by Brandywine Creek.

2. The extreme values of the intercepts (in the equations  $w=aQ^b$  and  $v=kQ^m$ ) for width and velocity, and the rate of change of width-depth ratio with dis-

charge extrapolated for data on Brandywine Creek, may reflect the failure of the channel to adjust to small discharges and indicate a relation of intercept to bank material.

3. Although the stream had some load before it issued from the glacier terminus, most of its load was provided by erosion of morainic debris, mudflow, and valley-train deposits in the reach below the glacier.

4. Analysis of a sample of mudflow and valley-train deposits showed that 40 percent of the mudflow material was less than 0.105 mm in comparison to 10 percent of the valley-train material, indicating removal of the finer fraction from the valley-train materials. Measurements demonstrated a systematic decrease in median diameter of the valley-train materials coarser than 4 mm with distance from the source. The change of 60 mm in median diameter for the valley train materials that took place over a distance of some 4,200 feet is attributed to selective transportation. Increases in median diameter such as those 1.7 and 25 miles below the glacier terminus occur at new sources of coarse material.

5. If one accepts the hypothesis of Wolman and Miller (1960) that the dominant process occurs with sufficient frequency and magnitude to cause most of the changes observed, and is neither the frequent event, which is less than the threshold value, nor the infrequent catastrophic event, then it follows that the slope forming discharge for White River channels in this reach lies between 200 and 500 cubic feet per second.

6. When conditions are such that antidunes are developed, much material is carried through the section without deposition. The rate of movement of antidunes, therefore, cannot be used to estimate the total bed load, as is sometimes done with dunes.

7. Measurements of competency in White River channels as well as data from Hjulström (1935) and Nevin (1946) suggest that for coarse materials the "sixth power law" should be the 7.8 power law. Logarithmic plotting shows that a line with a slope of 2.6 fits the White River data better than one with a slope of 2.0 (the "sixth power law" expressed in terms of linear dimensions rather than mass).

8. Evidence of the great amount of material transported by the White River is provided by the amount of erosion and deposition on the valley train. The average net elevation change for all profiles in 1958 was +1.2 feet and in 1959, -0.1 foot. The abundant discharge and high gradients of both East and West Emmons Creeks, together with the unique conditions described for West Emmons Creek, and the widening of the valley train, provided this abundant load during the summer of 1958. The high gradients of the Emmons Creeks combined with the lower flows of 1959

supplied materials to the valley train in greatly reduced quantities, so that erosion exceeded deposition.

9. The channel pattern changed from "meandering" to braided with the onset of high summer discharges and returned to "meandering" with the lower discharges of late summer and fall.

10. Flows in the "meanders" of the White River have relatively high Froude numbers. The "meanders" are a series of bends connected by short straight reaches of swift quiet flow; steeper gradients are usually associated with bends. It is apparent that there is no greater sinuosity associated with the meanders than with the anastomosing channels.

11. The wider the valley train and the farther it is downstream from a constriction, as exists at the junction of the Emmons Creeks, the more numerous the channels when braiding occurs.

12. Leopold and Wolman (1957) described the formation of a braided pattern in a flume channel by the deposition of a center bar which subsequently became an island. Observations of the White River show that a braided pattern may also result from the reoccupation of numerous old channels due to deposition within the main channel as well as to high flows which raise the water surface. This was an important mechanism in the formation and alteration of White River channel patterns. A pattern de-

veloped in this manner does not show an appreciable increase in slope for the divided reach as did the channel divided by the deposition of a center bar.

13. Both braided and meandering reaches can occur along White River in reaches where it is aggrading, poised, or degrading. The pattern alone does not conclusively define the regimen of the stream.

14. Braided channel patterns cannot be developed without bed load. The common element in all explanations of braiding appears to be a movement of bed load with local deposition within the channel, causing the diversion of flow from one channel into one or more other channels, or the deposition of channel bars and the development of islands. The rate of pattern change appears to be directly related to the amount of bed load.

15. The White River is a regrading stream showing adjustment between the variables of slope, channel cross section, discharge, load, and size of the bed and bank materials.

16. Observations of the White River show that although the regimen of the glacier has long-term effects in providing debris to the stream, the short-term effects of weather and runoff determine the rate of deposition and erosion, the hydraulic characteristics, and the pattern of the stream.



TABLE 7.—Characteristics of the White River channels, 1958-59

Channel No.	Date	Hour		Discharge (cfs)	Width (feet)	Mean depth (feet)	Area (square feet)	Mean velocity (feet per sec.)	Water surface slope (feet per foot)	Froude number	Maximum depth (feet)	Shape factor	Roughness factor $n$	Width: depth ratio	Distance above gage (feet)	Bottom condition <sup>1</sup>	Discharge measurement No.	Remarks
		From	To															
1	1958	1745	1900	134.0	18.0	1.44	25.9	5.17	---	0.76	2.5	1.7	---	12	---	Rough	1	(2) Boulders rolling through section.
2	6/30	0945	1205	124.0	18.0	1.47	26.4	4.70	---	.72	2.5	1.7	---	12	---	do	2	
3	7/1	1730	1855	183.5	25.0	.88	30.8	5.95	---	.90	1.4	1.6	---	40	400	do	3	
4	7/21	1730	1855	96.7	25.5	.87	21.7	4.45	---	.84	1.6	1.8	---	29	---	do	4	
5	7/21	1730	1955	12.8	13	.45	5.8	1.93	---	.51	1.0	2.2	---	29	---	---	5	
6	7/13	---	---	175	1.5	.078	117	---	0.014	.95	---	---	.021	19	1,300	Boulders	3a	
7	8/3	1500	1608	190.0	25	1.31	32.7	5.80	---	.89	1.9	1.4	---	19	---	---	4	
8	8/3	1500	1608	35.3	18	.58	10.4	3.29	.045	.74	0.8	1.4	.067	31	---	---	4	
9	8/7	1145	1230	79.5	22	.94	20.5	3.86	.028	.70	1.5	1.6	.062	23	800	do	5	
10	8/7	1145	1230	73.7	21	.84	17.7	4.16	.029	.80	1.1	1.3	.054	25	---	---	5	
11	8/7	1145	1230	85.3	18	.98	17.7	4.82	.023	.86	1.6	1.6	.046	18	---	do	5	Small anabranch.
12	8/15	1648	---	42.0	15.5	.72	11.2	3.76	.018	.78	1.1	1.5	.042	22	2,000	do	5a	Flow increased during measurement. <sup>3</sup>
13	8/15	1600	---	8.39	8.5	.44	3.75	2.24	.030	.59	1.5	1.1	.066	42	1,300	do	6	
14	8/18	1845	1930	26.7	10	.96	9.6	2.78	---	.50	1.5	1.6	---	10	---	---	6	
15	8/18	1845	1930	18.6	10.5	.51	5.4	3.46	---	.85	.8	1.6	---	20	---	---	6	
16	8/18	1845	1930	163.0	25	1.10	27.5	5.93	.020	.98	1.9	1.7	.038	23	900	Rough	6	(2)
17	8/19	0900	0920	23.7	15	.64	9.6	2.55	.029	.86	1.2	1.9	.074	23	---	---	7	
18	8/19	0900	0920	134.0	33	1.06	26.4	5.09	.032	.95	2.0	1.9	.054	24	900	Boulders	7	Boulders rolling through section. <sup>2</sup>
19	8/19	1415	1445	49.6	33	.56	18.6	2.67	.022	.85	.9	1.6	.056	58	---	---	8	Flow decreased after measurement.
20	8/19	1415	1445	211.0	29	1.21	35.0	6.03	.029	.90	1.7	1.4	.047	24	---	---	8	
21	8/19	1415	1445	2.1	6	.20	1.2	1.75	---	.43	.4	2.0	---	30	---	---	8	
22	8/19	1855	1920	32.9	16	.56	9.0	3.65	.027	.86	1.0	1.8	.045	29	1,500	Rough	9	Boulders rolling through section.
23	8/19	1855	1920	172.0	30	.99	29.7	5.77	.031	1.02	1.4	1.4	.045	30	---	---	9	
24	8/20	1545	1630	10.4	10	.50	5.0	2.03	---	.96	.9	1.8	.071	20	1,000	Small boulders	10	Much material moving along bottom. <sup>2,4</sup>
25	8/20	1545	1630	333.0	60	1.01	60.7	5.49	.032	.96	1.8	1.8	.049	60	---	---	10	
26	8/21	0855	0915	72.9	32	.637	20.0	3.65	.026	.81	1.4	2.2	.049	50	1,000	Boulders	11	Channel 26 losing flow to channel 27 during measurement. <sup>4</sup>
27	8/21	0855	0915	207.0	31	1.18	36.6	5.16	.045	.92	2.1	1.8	.062	26	900	Cobbles	11	Discharge shifting.
28	8/21	1030	1100	69.3	34	.87	20.8	3.34	.025	.63	1.2	1.4	.064	28	---	---	12	Small clear stream, little load. <sup>3</sup>
29	8/21	1030	1100	209.0	32	1.10	35.2	5.95	.027	1.00	1.6	1.4	.044	29	---	---	12	
30	8/25	---	---	.07	1.3	.05	.065	1.10	.024	.85	.14	2.7	.028	25	---	0.005-0.04' pavement	---	
31	8/25	---	---	.29	2.0	.13	.266	1.1	.013	.53	.16	1.2	.040	15	---	<.07' pavement	---	(3)
32	8/25	---	---	.02	.78	.04	.032	.5	.026	.63	.10	2.4	.056	19	---	<.05' pavement	---	(3)
33	8/25	---	---	.045	.95	.047	.045	1.0	.055	.81	.08	1.7	.045	20	---	Pavement	---	(3)
34	8/25	---	---	.23	1.50	.15	.226	1.0	.0092	.46	.18	1.2	.040	10	---	---	---	(3)
35	8/25	---	---	.17	2.75	.121	.333	.5	.004	.25	.19	1.6	.045	23	---	---	---	(3)
36	8/25	1230	---	.22	2	.093	.186	1.20	.0025	.69	.15	1.6	.013	22	---	---	---	(3)
37	8/25	---	---	.063	1.10	.064	.070	.9	.0057	.63	.11	1.7	.020	17	---	---	---	(3)
38	8/25	---	---	.005	.63	.027	.017	.3	.015	.32	.05	1.8	.055	23	---	<.05' ripples sand	---	(3)
39	8/25	---	---	.074	2.40	.102	.246	.3	.0017	.666	.17	1.7	.045	24	---	do	---	(3)
40	8/25	---	---	.25	4.10	.202	.830	.3	.0017	.6118	.34	1.7	.70	20	---	Sandy	---	(3)
41	8/26	---	---	72.5	16.8	0.93	15.6	4.67	.022	.85	1.4	1.5	.045	18	1,500	Rocky	13	Much material moving, uniform depth.
42	8/26	---	---	199.0	27	1.31	35.3	5.67	.032	.88	1.9	1.4	.056	21	---	do	13	Flow fast, turbulent.
43	8/26	---	---	9.6	9	.40	3.6	2.66	.029	.74	.6	1.5	.052	22	---	---	13	
44	8/26	---	---	2.9	.5	.27	1.35	2.15	.039	.72	1.5	1.8	.059	18	---	---	13	
45	8/26	---	---	130.0	24	1.13	27.2	4.78	---	.79	1.4	1.2	---	21	250	Cobbles, gravel	14	Flow fairly uniform.
46	8/28	0925	1010	147.0	28	1.11	31.1	4.73	---	.79	1.7	1.5	---	25	250	do	15	Flow fast, uniform, slightly muddy.
47	8/28	---	---	194.0	28	1.27	35.5	5.46	---	.84	1.7	1.3	---	22	220	do	16	Material moving along bottom.
48	8/28	---	---	183.0	28	1.24	34.8	5.27	---	.83	1.8	1.4	---	23	220	do	17	Flow fast and fairly uniform.
49	8/29	---	---	1.50	7.8	.11	.883	1.7	.004	.90	.24	2.2	.013	71	---	Sandy	---	Flow very smooth.
50	8/29	---	---	1.79	6.5	.19	1.26	1.3	.030	.53	.29	1.5	.068	34	---	Rocky	---	Chute on one side.
51	8/29	---	---	.76	4.5	.17	.76	1.0	.011	.43	.25	1.5	.048	26	---	Sandy, few rocks	---	White water.
52	9/3	1400	1435	93.7	24	1.09	26.1	3.59	---	.61	1.9	1.5	---	22	220	Cobbles, gravel	18	
53	9/3	1445	1520	96.2	24	1.13	27.2	3.53	---	.58	1.9	1.5	---	21	220	Cobbles, small boulders	19	Do.
54	9/30	1310	1350	108.0	22	1.25	27.5	3.96	---	.62	1.0	1.5	---	18	250	Cobbles, gravel boulders	20	Flow fairly uniform.
55	9/30	1425	1505	106.0	22	1.26	27.8	3.92	---	.61	1.9	1.5	---	18	250	Gravel, cobbles	21	Do.
56	11/25	---	---	30.7	15	.65	9.8	2.93	---	.64	1.4	1.2	---	23	50	Cobbles, gravel	21a	Clear, some shore ice, well frozen. <sup>5</sup>

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57	1959	1215	1225	5.39	8	.44	3.5	1.54	.41	8.	1.8	18	30	do	22	2 ft of snow on valley train.
58	5/27	1230	1255	48.82	23	.72	16.5	2.96	.61	1.0	1.4	32	200	do	22	Large kettles, water white to muddy.
59	6/18	1830	1845	135.3	19	.66	25.2	5.37	.83	2.1	1.6	42	150		23	No bed load movement, rough.
60	6/19	1840		57.48	28		18.4	3.12	.68	1.1	1.7	42	300		24	Flow swift, enlarged during p.m.
61	6/19	1100		53.53	16	.81	13.0	4.12	.81	1.0	1.2	20		Boulders	24	
62	6/19	1020		59.18	16	.01	16.2	4.58	.97	1.5	1.5	16			25	Flow swift.
63	6/30	1520	1534	100.30	19	1.15	21.9	4.58	.75	1.6	1.4	16	500		25	Boulders rolling.
64	6/30	1536	1543	100.2	7	1.47	3.3	3.09	.79	.8	1.7	15	240		26	Flow fast and uniform.
65	7/2	0930	1035	109.96	30	.86	25.85	.018	.81	1.5	.042	35			26	
66	7/2	1040	1050	7.79	14	.34	4.8	1.62	.49	.6	1.8	41			26	Chute along left edge draining kettles.
67	7/2	1100	1110	4.28	13	.34	4.45	.96	.29	.5	1.5	38			27	Flow swift, 1 in. material moving.
68	7/9	1400	1440	108.94	26	1.01	26.4	4.66	.82	1.4	.042	26	300		27	Underfit stream.
69	7/9	1440	1457	8.77	14	.41	5.8	1.51	.32	.6	.071	16			27	Clear stream.
70	7/9	1505	1512	3.16	6.5	.42	2.7	1.17	.32		1.4				28	
71	7/10	1530	1554	143.98	39	.94	36.7	3.93	.71	1.5	.054	42			29	Flow swift.
72	7/10	1715	1735	173.30	23	1.38	31.7	5.47	.82	1.9	.051	17			30	Do.
73	7/11	1820	1822	145.9	2.5	1.15	16	.92	.40	.3	1.9	16			30	Rocks to 0.5 ft rolling through section.
74	7/11	1822	1830	145.9	24.5	1.18	29.0	5.03	.82	1.8	.042	21			30	
75	7/11	1850	1920	84.1	31	.68	21.2	3.97	.85	1.1	.039	46			30	
76	7/13	0934	1603	118.8	20	1.07	21.4	5.55	.94	1.9	.028	19		Fairly fine gravel	31	Boulders rolling, shook ground.
77	7/15	1233		127.8	19.5	1.36	26.4	4.84	.94	1.8	.048	14	200		31	Flow swift.
78	7/15	1345		145.9	14.5	.88	12.7	3.69	.69	1.4	.047	16			32	
79	7/17	1730		173.9	24	1.00	34.0	5.17	.91	1.6	.030	34	250		32	Flow swift.
80	7/17	1833		78.5	23	1.01	23.2	3.38	.59	1.8	.046	23			33	6-in. rocks moving, one was 1 ft.
81	7/19	1435	1453	153.8	24	1.12	26.8	5.73	.94	1.5	.041	22	250	Sandy	33	Flow shifted somewhat into ch.
82	7/19	1454	1514	211.9	23	1.18	27.1	7.83	1.27	2.3	.033	20		Sandy	33	82 during measurement.
83	7/21	1530	1548	298.0	29	1.13	32.7	9.12	.51	1.8	.021	26				Material 8-10 in. moving.
84	7/21	1714	1728	16.0	16	.41	6.5	2.4	.66	.6	1.5	39				
85	7/21	1755	1802	18.0	11	.53	5.8	3.05	.74	.9	.040	21				
86	7/21	1805	1811	26.0	16	.51	8.2	3.48	.86	1.8	.032	31				
87	7/24	1543	1630	432.0	28	2.08	58.2	7.43	.91	3.2	1.5	13				
88	7/30	1617	1655	238.0	28	1.05	29.4	8.10	1.38	2.2	.046	27				
89	7/31	0945	1055	223.0	29	1.47	42.5	5.25	.76	2.8	1.9	20				
90	7/31	1415	1545	281.0	29	1.69	49.1	5.73	.78	3.0	.057	17				
91	7/31	1640	1715	170.0	30	1.60	48.1	5.61	.78	2.9	.066	19				
92	8/1	1855	1930	273.0	30	1.53	46.0	5.92	.84	2.9	.046	20				
93	8/3	1800	1848	182.0	28	1.24	34.6	5.27	.83	2.0	.036	25				
94	8/5			96	7.2	.211	1.82	.631	.24	.28		35				
95	8/5												300			
96	8/5	1830	1855	177.0	31	1.08	33.5	5.21	.88	2.2	.048	29				
97	8/9	1449	1605	201.0	28	1.34	27.5	5.37	.82	2.0	.063	21				
98	8/13	1135	1200	55.1	11.2	.94	10.5	5.25	.95	1.7	.085	12				
99	8/13	1300	1530	180.0	24	1.35	32.3	5.23	.79	1.8	.042	18				
100	8/17	1540	1600	170.0	20	1.48	29.6	5.58	.81	2.5		14				
101	8/24	1022	1038	133.0	24	1.12	26.8	4.96	.83	1.7	.045	22				
102	8/24	1108	1123	8.0	9	.37	3.3	2.51	.73	.6	.018	24				
103	8/24	1410	1502	143.0	32.5	.84	30.4	4.76	.86	1.4	.070	34				
104	8/24	1600	1620	14.0	11	.53	5.8	2.46	.59	.8	.054	21				
105	8/28	1055	1155	149.0	27	1.10	29.8	5.17	.87	2.0	1.8	24				
106	8/28	1300	1350	165.0	32	1.00	32.1	5.14	.91	1.9	.042	32				
107	9/22	1230	1305	86.0	20	1.23	24.7	3.48	.55	2.0	.018	16				
108	9/22	1310	1315	1.7	6	.38	2.3	3.74	.47	.5	1.3	16				
109	11/10	1210	1225	19.0	16	.58	8.6	2.20	.53	.7	1.3	30				
110	11/10	1230	1245	7.0	10	.33	3.3	2.11	.65	.5	1.5	30				
111	11/27	1235	1305	39.0	15	.93	14.0	2.78	.51	1.7	1.8	16				
112	11/27	1305	1313	3.0	6	.48	2.9	1.03	.26	.7	1.5	12				

<sup>1</sup> Largest material present is given as bottom condition.

<sup>2</sup> Water temperature measured and within 35°-39° range.

<sup>3</sup> Pigmy current meter used.

<sup>4</sup> Measurement of load made in reach at time of discharge measurement. See table 3 for data on load measurement.

<sup>5</sup> Water temperature measured and within 32°-34° range.

<sup>6</sup> Based on float velocities.

TABLE 8.—*Bed load of the White River*  
 [Asterisk (\*) indicates *L*, length; *W*, width; *H*, height]

Date, time	Channel width (feet)	Depth (feet)		Tractive force	Maximum velocity (fps)		Mean velocity (fps)	Slope (feet per foot)	Load			Discharge (cfs)	Remarks
		Max.	Mean		Point	Float			Dimensions* 5 largest boulders (feet)			(Lbs per min per ft)	
									(L)	(W)	(H)		
Screen-Caught Samples													
7/8/58, 1730 hrs.....		1.1		0.79	18.2	9.4		0.012	0.46	0.29	0.21		Antidunes 0.05 ft.
									.32	.29	.18		
									.32	.25	.21		
									.32	.25	.18		
									.29	.25	.16		
7/8/58, 1730 hrs.....		1.2		.57	18.2	9.4		.012	.37	.33	.16	20	Crest of antidune 12 sec; sample 0.05 ft screen.
									.37	.25	.19		
									.25	.23	.15		
									.29	.21	.17		
									.25	.21	.17		
7/8/58, 1730 hrs.....		.9		.53	18.2	9.4		.012	.38	.34	.25	50	Trough of antidune 12 sec; sample; 0.05 ft screen.
									.38	.33	.21		
									.29	.26	.21		
									.39	.25	.16		
									.29	.25	.16		
8/3/58, 1500-1600 hrs.....	18	.8	.58	1.53	4.5	5.0	3.47	.045	.36	.32	.25	35	Channel 8; discharge measurement 4; 0.05 ft screen.
									.27	.22	.17		
									.25	.22	.16		
									.31	.18	.13		
									.13	.10	.07		
8/5/58, 1500-1600 hrs.....	25	1.7	1.3	2.04	6.8	6.5	5.8	.025	.40	.37	.20	189	Channel 7; discharge measurement 4; 0.05 ft screen.
									.50	.36	.19		
									.47	.36	.22		
									.48	.35	.31		
									.42	.35	.30		
8/3/58.....		1.2	2.7	1.03	4.2	9.0		.023	.35	.28	.20		0.05 ft screen.
									.36	.25	.11		
									.29	.25	.16		
8/3/58.....		1.05	2.66	1.90	4.8	7.1		.029	.32	.28	.22	7	0.05 ft screen.
									.24	.24	.18		
									.27	.23	.18		
									.25	.22	.19		
									.30	.19	.17		
8/7/58, 1540 hrs.....	27	1.1	.7	1.22	17.5	8.6		.027	1.3	1.1	1.1	114	0.175 screen.
									1.3	1.1	.9		
									1.3	.9	.9		
									1.1	.8	.8		
									1.1	.8	.6		
8/9/58, 1730-1800 hrs.....		1.4	2.87	2.88	6.8	10		.033	.7	.7	.5		0.175 ft screen; bottom of channel smooth and gravelly.
									.9	.6	.5		
									.8	.6	.4		
									.7	.6	.4		
									.7	.6	.3		
8/15/58, 1648 hrs.....	15.5	1.1	.72	.81	5.8	16.7	3.76	.018	.32	.25	.15	42	Channel 12; 0.05 ft screen.
									.31	.23	.20		
									.21	.18	.13		
									.23	.18	.17		
									.19	.18	.18		
8/20/58, 1545-1630 hrs.....	60	1.3	1.01	2.04	8.82	10.2	5.6	.019	.8	.7	.45	44	330 Load taken in main channel after Q measurement.
									.8	.6	.50		
									.65	.6	.45		
									.65	.57	.30		
									.68	.55	.40		
8/21/58, 0835-0915 hrs.....	31	2.1	1.18	1.91	7.6	8.9	5.72	.026	.8	.7	.4	82	206 Suspect bank erosion upstream
									.7	.7	.5		
									.6	.5	.3		
									.45	.45	.27		
									.45	.45	.15		
8/21/58, 1115 hrs.....	32	1.1	1.1	1.85	5.8	16.7	5.95	.027	.85	.55	.40	107	209
									.70	.43	.35		
									.53	.35	.30		
									.45	.35	.35		
									.45	.35	.30		
7/13/59, 0934 hrs.....	20	1.9	1.07	1.24	7.9	9.2	5.55	.18	.9	.75	.7	119	Channel 76; boulders rolling through section about sta. 9; shook ground.
									.8	.7	.6		
									.9	.6	.5		
									.9	.6	.5		
									.7	.6	.6		
7/9/59, 1645 hrs.....	26	1.3	1.01	1.07	5.0	8.6	4.66	.017	None taken			18	109 Sta. 21-23; channel 68; material moving was small.
7/9/59, 1710 hrs.....	26	1.0	1.01	1.07	5.9	8.6	4.66	.017	.6	.5	.4	32	109 Sta. 23-25; channel 68; 0.04 ft screen.

See footnotes at end of table.



TABLE 8.—Bed load of the White River—Continued

Date, time	Channel width (feet)	Depth (feet)		Tractive force	Maximum velocity (fps)		Mean velocity (fps)	Slope (feet per foot)	Load			Discharge (cfs)	Remarks
		Max.	Mean		Point	Float			Dimensions* largest boulders (feet)				
									(L)	(W)	(H)		
Screen-Caught Samples—Continued													
8/10/59, 1720-1725 hrs.		1.5	20.94	1.03	8.1	10		0.011	1.0	0.8	0.7		Screen leg broken; only 2 samples taken.
8/14/59		1.0	2.63	0.59	5.4	16.3		.008	1.4	.8	.7		0.175 ft screen.
									1.2	.8	.8		
									1.1	.8	.7		
									1.0	.7	.5		
									.9	.8	.6		
Samples on Bars													
6/20/58, 1630 hrs.					19.3	10.6		0.025	1.7	1.3	1.1		
									1.2	1.0	.8		
									1.2	.9	.8		
									1.3	.8	.8		
									1.1	.8	.8		
6/20/58, 1630 hrs.					17.4	8.5		.025	2.0	1.7	1.5		
									1.5	1.3	1.0		
									1.4	1.3	1.1		
									1.2	1.2	1.0		
									1.4	1.0	1.0		
6/23/58, 1500 hrs.					11.0	12.6		.022	1.7	1.5	.9		
									2.1	1.3	1.0		
									1.6	1.3	1.1		
									1.7	1.2	.9		
									1.3	1.2	1.0		
6/25/58, 1900 hrs.					17.0	8.0		.176	1.9	1.7	1.7		
									1.9	1.6	1.5		
									1.6	1.6	1.6		
									2.1	1.5	1.4		
									1.9	1.5	1.5		
8/3/58		1.8	21.13	2.04	7.5	7.7		.029	.45	.37	.25		Caught by hand.
									.43	.35	.25		
									.25	.20	.18		
8/5/58, 1430 hrs.	58	3	1.1	3.85	17.0	8.0		.056	1.4	1.2	.9		Not too much moving. Most moved out when pushed and continued to move.
									1.1	1.1	1.0		
									1.6	1.0	1.0		
									1.1	1.0	1.0		
									1.1	.9	.8		
8/6/58, 1030 hrs.	27	2.4	2.0	5.36	17.8	9.0		.043	1.6	1.6	1.3		Measured on bar head, then moved out.
									1.6	1.3	1.2		
									1.3	1.2	.9		
									1.6	1.1	1.1		
									1.5	1.1	1.0		
8/6/58, 1210 hrs.	43	2.5	1.3	2.52	19.2	10.5		.031	1.3	1.1	.7		Smooth bottom washes out under foot. Material may be undermined.
									1.2	1.1	.9		
									1.4	1.0	.6		
									1.2	1.0	.9		
									1.2	1.0	.6		
8/8/58	46	2.5	1.2	2.23	18.0	9.2		.030	2.8	1.8	1.5		
									2.1	1.8	1.2		
									1.6	1.2	1.1		
									1.3	1.2	1.1		
									2.3	1.0	.9		
8/9/58	25				18.0	9.2		.050	1.8	1.5	1.0		
									1.5	1.4	1.0		
									2.4	1.2	1.0		
									2.0	1.2	1.1		
									1.6	1.0	.7		
8/13/58, 1715 hrs.	60	2	1.3	1.54	19.0	10.3		.019	1.7	1.3	.7		
									1.3	1.2	1.0		
									1.8	1.1	1.0		
									1.5	1.1	1.0		
									1.5	1.0	.7		

1 Velocity read from graph. Float velocity=1.15 point velocity.

2 Mean depth computed using mean depth=0.625 maximum depth.

3 Tractive force based on calculated mean depth.

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